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TECHNICAL REPORT NO. 10561

FAMSNUB, A COMPUTER PROGRAM  
FOR DETERMINING THE FREQUENCIES AND  
MODE SHAPES OF NONUNIFORM BEAMS



MECHANICAL BRANCH  
AUTOMOTIVE COMPONENTS DIVISION

September 1969

by John L. McFarland

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## ABSTRACT

FAMSNUB is a digital computer program, written in single precision FORTRAN IV for the Philco-Ford/GE 265 time-sharing system, to determine the eigenfrequencies and eigenmodes of vibration in pure bending of loaded, piece-wise continuous single-span beams supported in any one of the ten possible combinations of the following end conditions: free, guided, hinged and clamped. Continuously variable beam properties may be accommodated by appropriate subdivision of the span. The distributed mass of the beam itself is taken into account by means of field transfer matrices; this represents an advance over the usual lumped-parameter approach. To preserve numerical accuracy at the higher mode values, a modified transfer matrix method is employed. Verification solutions have been obtained on the IBM 360-50/65 using the double precision facility.

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## FOREWORD

This report documents results obtained from project work on task title "In-House Initiated Lab R & D" (DA Proj. No. 1T061101A91A/Task Area 00/Work Unit No. 205EH) authorized by Dr. E.N. Petrick, USATACOM Chief Scientist/Technical Director of Laboratories. The project is intended to strengthen in-house research capabilities and provide a methodology for dealing with practical problems. The author extends his sincerest appreciation to Dr. Petrick for the opportunity to perform this work and to his senior colleague, Mr. Douglas J. Hackenbruch for the encouragement and assistance he has provided during all phases of this project.



## INTRODUCTION

This work was initiated after a literature search revealed that no generally applicable computerized solution for beam vibration eigenvalues had yet been developed which took account of the continuity of beam properties. The analytical development contained herein has been applied to piece-wise continuous beams taking account of bending only. Extensions which would include continuous variation of properties, the effects of shear deflection and rotary inertia, intermediate support conditions and response to forcing functions are quite feasible.

Conventional numerical methods of determining natural frequencies of transverse beam vibration generally employ variations of lumped-parameter techniques. To account in a more refined way for the distributed mass of the elastic system, field transfer matrices, which will be developed in the following section, have been incorporated into FAMSNUB and combined with point transfer matrices to accommodate discrete loading. In addition to discrete loading, step-wise variations can be accommodated in: uniformly-distributed loading, beam material density, elastic modulus and section modulus.

The source language employed should provide greater time-sharing system compatibility insofar as no use is made of subscripted subscripts, mixed modes, variable name lengths exceeding six characters nor of the double precision facility. To accommodate users of time-sharing systems, program length limitations have been observed by subdividing FAMSNUB into 5 saved items dealing with the conditioning of input data, eigenfrequency calculation (3 parts) and the computation of normal modes. This has been accomplished through the employment of intermediate permanent file storage and the \$USE monitor line.

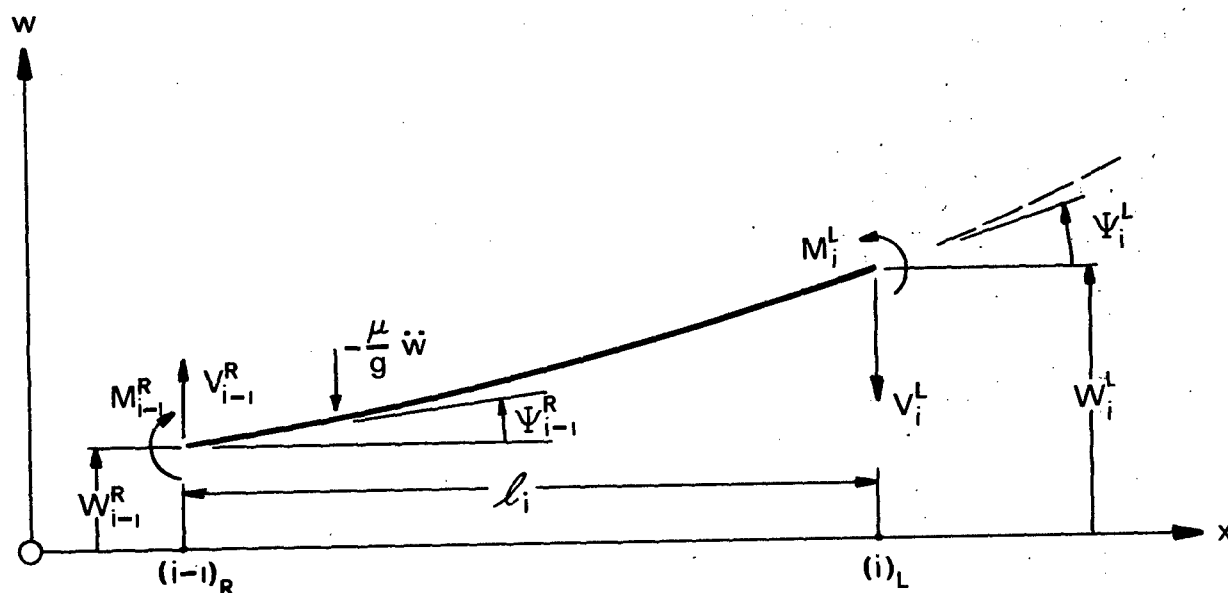
The considerable numerical magnitude difficulties encountered in evaluating  $2 \times 2$  frequency determinants, which involve taking the difference between quantities which are several orders of magnitude larger than the difference itself, are sidestepped by an adaptation of a method developed by Pestel & Leckie (1), pp. 204-213.

FAMSNUB commences with the first non-zero frequency and proceeds continuously to the mode(s) of interest. However, a start may occur at any frequency to investigate a resonance condition. If the eigenmodes are also desired, a printout can be obtained in four-column format of deflection amplitude, maximum bending stress and average transverse shear stress, together with the corresponding distance along the beam (Figure 27).

## OBJECT

Develop a working method for computing the eigenvalues and eigenmodes of transverse beam vibration, taking account of the distributed beam mass in preference to employing a lumped-parameter approach.

## PROBLEM DESCRIPTION



Forces and Deflections for Beam Element

Figure 1

### Field and Point Transfer Matrices

Consider an initially straight, piece-wise continuous beam performing free transverse vibrations. Figure 1 shows an isolated beam element of length  $l_i$  between points  $(i-1)$  and  $(i)$ , having continuous elastic properties and a constant rate of inertia loading  $-\frac{\mu}{g} \ddot{w}$ .  $\mu$  is the non-reactive loading per unit length due to beam self-weight and continuous loading. The origin of the coordinate plane, in which all vibration is assumed to occur,

is to be considered as coincident with the left-hand support. The sign convention is also revealed by Figure 1, which shows positive, instantaneous values of the state vector components: deflection  $w$ , slope  $\Psi$  (in radians), bending moment  $M$  and shear force  $V$ . The uniform bending stiffness for this length is  $EI$ , the product of the elastic modulus and the second moment of area of the cross-section.

From simple bending theory, the fourth partial derivative of  $w(x,t)$  with respect to  $x$  is proportional to the rate of loading, thus

$$EI \frac{\partial^4 w(x,t)}{\partial x^4} = -\frac{\mu}{g} \ddot{w} \quad (1)$$

For a solution which permits eigenfrequency determination we write  $w$  as the product of two single-variable functions

$$w(x,t) = W(x) \sin \omega t \quad (2)$$

where  $W(x)$  is the dynamic amplitude and  $\omega$  is the simple harmonic circular frequency; substituting into (1)

$$EI \frac{d^4 W}{dx^4} = \frac{\mu}{g} W \omega^2 \quad (3)$$

which becomes

$$\frac{d^4 W}{dx^4} - m^4 W = 0 \quad (4)$$

where

$$m = \sqrt[4]{\frac{\mu \omega^2}{gEI}} \quad (5)$$

Rewriting (4) in LaPlace transform notation

$$p^4 W(p) - p^3 W_0 - p^2 W_0' - p W_0'' - W_0''' - m^4 W(p) = 0 \quad (6)$$

where  $W(p) = \int_0^\infty e^{-px} f(x) dx$   $W_0^n = \left. \frac{d^n W}{dx^n} \right|_{x=0}$

solving  $W(p) = \frac{1}{p^4 - m^4} \left[ p^3 W_0 + p^2 W_0' + p W_0'' + W_0''' \right] \quad (7)$

where  $W_0 = W_{i-1}^R$ ;  $W_0' = \Psi_{i-1}^R$ ;  $E_i l_i W_0'' = M_{i-1}^R$ ;  $E_i l_i W_0''' = V_{i-1}^R$

Making the appropriate substitutions and applying inverse transforms:

$$W(x) = W_i^L = \frac{\cos mx + \cosh mx}{2} W_{i-1}^R + \frac{\sin mx + \sinh mx}{2m} \Psi_{i-1}^R \\ + \frac{\cosh mx - \cos mx}{2E_i l_i m^2} M_{i-1}^R + \frac{\sinh mx - \sin mx}{2E_i l_i m^3} V_{i-1}^R \quad (8)$$

$$W'(x) = \Psi_i^L = \frac{m(\sinh mx - \sin mx)}{2} W_{i-1}^R + \frac{\cos mx + \cosh mx}{2} \Psi_{i-1}^R \\ + \frac{\sin mx + \sinh mx}{2E_i l_i m} M_{i-1}^R + \frac{\cosh mx - \cos mx}{2E_i l_i m^2} V_{i-1}^R \quad (9)$$

$$W''(x) = M_i^L = \frac{E_i l_i m^2 (\cosh mx - \cos mx)}{2} W_{i-1}^R + \frac{E_i l_i m (\sinh mx - \sin mx)}{2} \Psi_{i-1}^R \\ + \frac{\cos mx + \cosh mx}{2} M_{i-1}^R + \frac{\sin mx + \sinh mx}{2m} V_{i-1}^R \quad (10)$$

$$Z_i^L =$$

$$F_i$$

$$Z_{i-1}^R$$

$$\begin{bmatrix} W \\ \psi \\ M \\ V \end{bmatrix}_i^L = \begin{bmatrix} \frac{\cosh mx + \cosh mx}{2} & \frac{\sinh mx + \sinh mx}{2m} & \frac{\cosh mx - \cosh mx}{2EI m^2} & \frac{\sinh mx - \sinh mx}{2EI m^3} \\ \frac{m(\sinh mx - \sinh mx)}{2} & \frac{\cosh mx + \cosh mx}{2} & \frac{\sinh mx + \sinh mx}{2EI m} & \frac{\cosh mx - \cosh mx}{2EI m^2} \\ \frac{EI m^2(\cosh mx - \cosh mx)}{2} & \frac{EI m(\sinh mx - \sinh mx)}{2} & \frac{\cosh mx + \cosh mx}{2} & \frac{\sinh mx + \sinh mx}{2m} \\ \frac{EI m^3(\sinh mx + \sinh mx)}{2} & \frac{EI m^2(\cosh mx - \cosh mx)}{2} & \frac{m(\sinh mx - \sinh mx)}{2} & \frac{\cosh mx + \cosh mx}{2} \end{bmatrix}_{i-1}^R \begin{bmatrix} W \\ \psi \\ M \\ V \end{bmatrix}_{i-1}^R$$

Transfer Matrix for an Elastic Field

Figure 2



$$W'''(x) = V_i^L = \frac{E_i l m^3 (\sin mx + \sinh mx)}{2} W_{i-1}^R + \frac{E_i l m^2 (\cosh mx - \cos mx)}{2} \Psi_{i-1}^R \\ + \frac{m (\sinh mx - \sin mx)}{2} M_{i-1}^R + \frac{\cos mx + \cosh mx}{2} V_{i-1}^R \quad (11)$$

Hence the relation of the state vectors  $Z_{i-1}^R$  and  $Z_i^L$  on either side of an elastic field of length  $x$  can be stated as

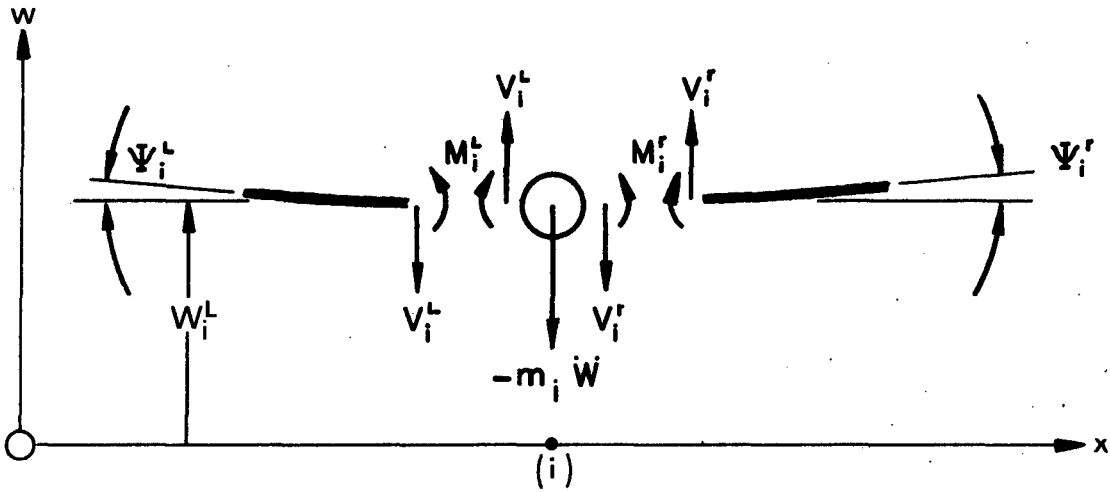
$$Z_i^L = F_i \cdot Z_{i-1}^R \quad (12)$$

Equation (12) can be represented in matrix notation as shown in Figure 2.  $F_i$  is called the field transfer matrix which, by taking the limit as  $m$  approaches zero, reduces to the well-known transfer matrix for an elastic, massless field as shown in Figure 3.

$$\begin{bmatrix} W \\ \Psi \\ M \\ V \end{bmatrix}_i^L = \begin{bmatrix} 1 & x & \frac{x^2}{2EI} & \frac{x^3}{6EI} \\ 0 & 1 & \frac{x}{EI} & \frac{x^2}{2EI} \\ 0 & 0 & 1 & x \\ 0 & 0 & 0 & 1 \end{bmatrix}_i \cdot \begin{bmatrix} W \\ \Psi \\ M \\ V \end{bmatrix}_{i-1}^R$$

Transfer Matrix for a  
Massless, Elastic Field

Figure 3



Free Body Diagram for the Point Mass  $m_i$   
Figure 4

$$Z_i^R = P_i \cdot Z_i^L$$

$$\begin{bmatrix} W \\ \Psi \\ M \\ V \end{bmatrix}_i^R = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ m\omega^2 & 0 & 0 & 1 \end{bmatrix}_i \cdot \begin{bmatrix} W \\ \Psi \\ M \\ V \end{bmatrix}_i^L$$

Transfer Matrix for a Point Mass  
Figure 5

With reference to the free body diagram for the point mass  $m_i$  situated at point (i) as shown in Figure 4, the state vector components on either side of  $m_i$  are related as follows:

$$W_i^R = W_i^L$$

$$\Psi_i^R = \Psi_i^L$$

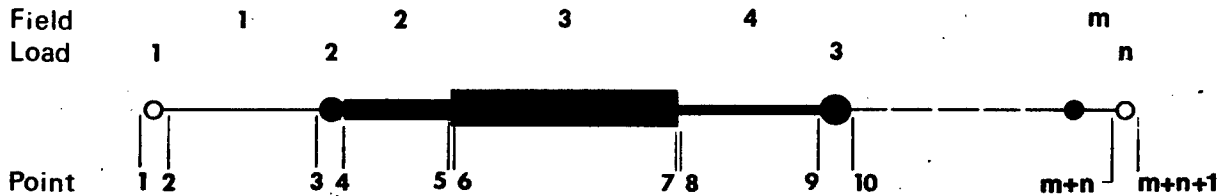
$$M_i^R = M_i^L$$

$$V_i^R = V_i^L + m_i W_i \omega^2 \quad (13)$$

where  $\omega$  is the harmonic circular frequency. Thus a relationship similar to (12) may be written

$$Z_i^R = P_i Z_i^L \quad (14)$$

where  $P_i$  is the point transfer matrix. Equation (14) may be written in matrix notation as shown in Figure 5.



General, Piece-wise Continuous Beam

Figure 6

The relation between the state vectors  $Z_{m+n+1}$  and  $Z_1$  at each end of the generalized beam shown in Figure 6, having  $m$  uniform elastic fields and  $n$  point loads will be of the form

$$\begin{aligned} Z_{m+n+1} &= P_1 \cdot F_1 \cdot P_2 \cdot F_2 \cdot F_3 \cdot F_4 \cdot \dots \cdot F_m \cdot P_n \cdot Z_1 \\ &= U \cdot Z_1 \end{aligned} \quad (15)$$

where the intermediate state vectors will have been eliminated by multiplication of the transfer matrices  $P_i$  and  $F_i$  to form the overall transfer matrix  $U$ . For the idealized end conditions treated in this report two columns of  $U$  can be ignored, owing to the two zero components of  $Z_1$ ; also two components of  $Z_{m+n+1}$  will be zero, allowing the formation of a  $2 \times 2$  determinant  $D$ .

A simple beam may be treated algebraically, in which case the coefficients of  $D$  would involve functions of  $\omega$ . Solutions for  $\omega_n$  would be obtained from the eigenfrequency condition  $D=0$ . Realistic problems demand a numerical approach in which  $\omega_n$  is surmised and the value of  $D$  obtained. If the latter is plotted against  $\omega$ , the eigenvalues can be bracketed with reasonable accuracy. However, using only eight digits may give rise to precision problems at low frequencies even for simple beams. A more generally applicable numerical approach has been adapted from Pestel and Leckie, (ref. 1, pp. 204-213), referred to therein as the "modified transfer matrix method". This method avoids evaluating  $U$  and hence  $D$  by directly obtaining the two (ideally) zero components of  $Z_{m+n+1}$ . One of the two nonzero components of  $Z_1$  is normalized to unity, the other being arbitrarily chosen. For a given estimate of eigenfrequency, a single-valued frequency function  $R(\omega)$  can be obtained after two iterations. The first iteration establishes the correction to be applied to the non-normalized component of  $Z_1$  as the arithmetic mean of the two ideally zero components of  $Z_{m+n+1}$ . These latter components are again obtained and, at the eigenfrequency condition, will be zero; generally, however, their difference, which is termed  $R(\omega)$ , will be nonzero and can be considered analogous to the value of  $D$  at a general  $\omega$ . This method has the advantage that numerical difficulties arising from evaluating  $D$ , where the difference is several orders of magnitude smaller than the minuend and subtrahend, have been eliminated. This process was the principal error source, and, using the modified method, high eigenfrequencies of complex beams can be determined with reasonable accuracy without recourse to the double precision facility.

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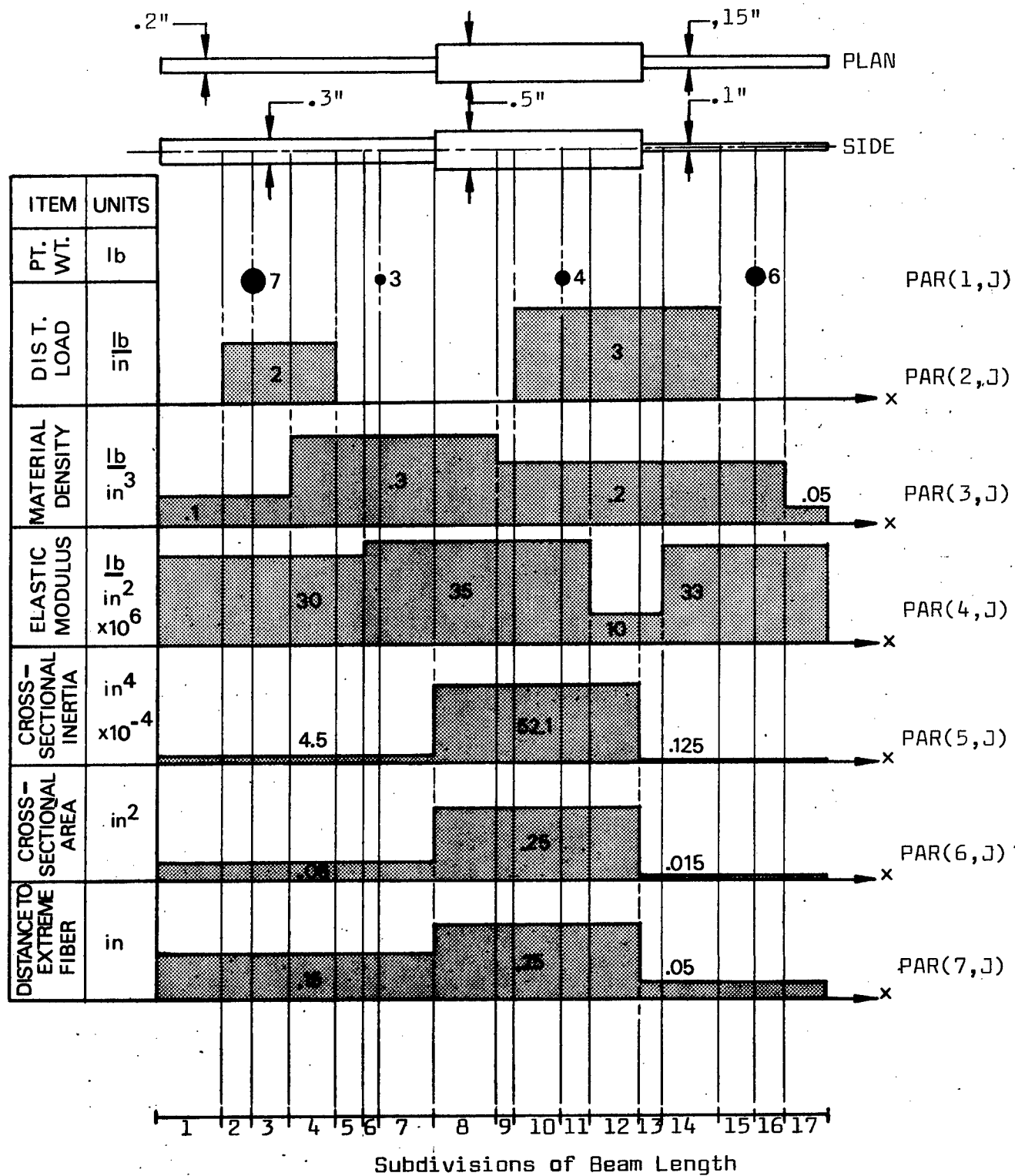


FIGURE 7

### Conditioning of Input Data

In its time-sharing form, as mentioned in the Introduction, FAMSNUB appears in 5 separate parts or saved items: EMB10 which conditions the input data; EMB15 which in conjunction with EMB20 and EMB21 computes the eigenfrequencies and state vectors; and EMB25 which computes the eigenmodes and stress values. The following description of FAMSNUB is made with reference to these time-sharing component items, which will in turn be illustrated by a sample problem. This problem very nearly exhausted the core storage capacity of the Philco-Ford system at the time of writing.

The beam dealt with is shown in Figure 7 opposite. Seven parameters  $PAR(I,J)$ ,  $I=1,7$  are considered to describe fully the discrete and distributed loading systems and the beam's material and geometrical properties as follows:

$PAR(1,J)$ ,  $J=1,4$  are point weights of 7,3,4 and 6 pounds positioned 5.1,11.9,21.8 and 32.1 inches respectively from the left-hand support.

$PAR(2,J)$ ,  $J=1,2$  are uniformly-distributed loadings of 2 and 3 lb/in., applied over the lengths shown.

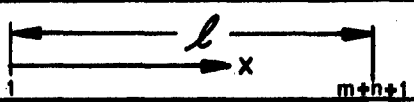

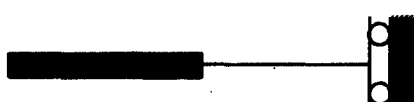


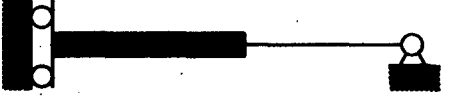
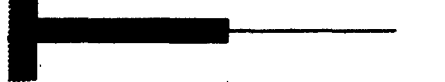

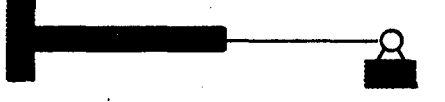
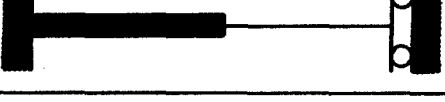

$PAR(3,J)$ ,  $J=1,4$  are the values of material density.

$PAR(4,J)$ ,  $J=1,4$  are the values of the elastic modulus.

$PAR(5,J)$ ,  $J=1,3$  are the values of area moment about the neutral axis.

$PAR(6,J)$ ,  $J=1,3$  are the values of cross-sectional area.

$PAR(7,J)$ ,  $J=1,3$  are the distances from the neutral axes of the various sections to the outermost bending fiber.

NSUPP		SUPPORT CONDITIONS	STATE VECTORS	
			$z_1$	$z_{m+n+1}$
1		FREE-FREE	$\begin{matrix} 1 \\ \lambda \\ 0 \\ 0 \end{matrix}$	$\begin{matrix} - \\ 1 \\ 0 \\ 0 \end{matrix}$
2		FREE-GUIDED	$\begin{matrix} 1 \\ \lambda \\ 0 \\ 0 \end{matrix}$	$\begin{matrix} - \\ 0 \\ 1 \\ 0 \end{matrix}$
3		FREE-HINGED	$\begin{matrix} 1 \\ \lambda \\ 0 \\ 0 \end{matrix}$	$\begin{matrix} 0 \\ 1 \\ 0 \\ -1 \end{matrix}$
4		GUIDED-GUIDED	$\begin{matrix} 1 \\ 0 \\ \lambda \\ 0 \end{matrix}$	$\begin{matrix} - \\ 0 \\ 1 \\ 0 \end{matrix}$
5		GUIDED-HINGED	$\begin{matrix} 1 \\ 0 \\ \lambda \\ 0 \end{matrix}$	$\begin{matrix} 0 \\ 1 \\ 0 \\ -1 \end{matrix}$
6		CLAMPED-FREE	$\begin{matrix} 0 \\ 0 \\ \lambda \\ 1 \end{matrix}$	$\begin{matrix} - \\ 1 \\ 0 \\ 0 \end{matrix}$
7		HINGED-HINGED	$\begin{matrix} 0 \\ \lambda \\ 0 \\ 1 \end{matrix}$	$\begin{matrix} 0 \\ 1 \\ 0 \\ -1 \end{matrix}$
8		CLAMPED-HINGED	$\begin{matrix} 0 \\ 0 \\ \lambda \\ 1 \end{matrix}$	$\begin{matrix} - \\ 1 \\ 0 \\ -1 \end{matrix}$
9		CLAMPED-GUIDED	$\begin{matrix} 0 \\ 0 \\ \lambda \\ 1 \end{matrix}$	$\begin{matrix} - \\ 0 \\ 1 \\ 0 \end{matrix}$
10		CLAMPED-CLAMPED	$\begin{matrix} 0 \\ 0 \\ \lambda \\ 1 \end{matrix}$	$\begin{matrix} 0 \\ 1 \\ 0 \\ -1 \end{matrix}$

End Constraints and State Vectors

Figure 8



EMB10R	NSS	NOSUP	NMINT	NMSHP	NOINTP	G	BMLGTH	NPAP(I)
100	17	3	5	5	10	386.	36. {4 5 4 4 3 3 3}	
105		{3.4		1.7		2.1	2.4	1.4
110	SS(I)	{3.0		3.4		.8	2.7	2.1
115		{1.1		3.1		1.9	1.7	2.2
120	NSUPP(I)	{2 7 10}	{1	2	3	4	5}	{1 2 3 4 5}
125	.7000E	01	3					
130	.3000E	01	7					
135	.4000E	01	11					
140	.6000E	01	16					
145	.0000E	00	1					
150	.2000E	01	2					
155	.0000E	00	5					
160	.3000E	01	10					
165	.0000E	00	15					
170	.1000E	00	1					
175	.3000E	00	4					
180	.2000E	00	9					
185	.5000E-01	17						
190	.3000E	08	1					
195	.3500E	08	6					
200	.1000E	08	12					
205	.3300E	08	14					
210	.4500E-03	1						
215	.5210E-02	8						
220	.1250E-04	13						
225	.6000E-01	1						
230	.2500E	00	8					
235	.1500E-01	13						
240	.1500E	00	1					
245	.2500E	00	8					
250	.5000E-01	13						

MINT(I)      MSHP(I)

PAR(1,J)      PAR(2,J)      PAR(3,J)      PAR(4,J)      PAR(5,J)      PAR(6,J)      PAR(7,J)

Initial Data Set EMB10R

Figure 9

The NOSUP=10 different support conditions are illustrated in Figure 8, each condition corresponding to a value of the subscripted variable NSUPP. The state vectors  $Z$  and  $Z_{m+n+1}$  are given in forms corresponding to the requirements of the modified transfermatrix method, i.e. the two non-zero components of  $Z_1$  appear as unity and  $\lambda$ , which represents the initial, arbitrary choice of slope or bending moment as the case may be. For the sample problem, three support conditions are chosen: NSUPP(1)=2 (free-guided), NSUPP(2)=7 (hinged-hinged) and NSUPP(3)=10 (clamped-clamped). The order of support condition can in general be accounted for by taking the origin of  $x$  at the appropriate end; thus if for the sample problem the initial support condition had been guided-free, the right-hand end would have been taken as the origin. It would be feasible for all ten conditions to be included in one run on a batch system, but for time-sharing purposes the saved items EMB15/EMB20/EMB21 will terminate if the eigenmodes and stress values are desired, and EMB25 will have to be run separately. The input data are contained in file EMB10R as shown in Figure 9. EMB10 is shown in Figure 10.

<u>EMB10R</u> <u>Lines</u>	<u>Read by</u> <u>EMB10 Lines</u>	<u>EXPLANATION</u>
100	4-5	Line 100 will always contain 14 items, regardless of problem complexity, as follows:  NSS is the number of subspans into which the beam is naturally subdivided by the point loads and the discontinuities introduced by the distributed quantities. This is illustrated in Figure 7, where the finer, vertical lines denote the intervals.  NOSUP is the number of support conditions to be examined.  NMINT is the number of modes for which the eigenfrequency is desired.

		<p>NMSHP (<math>\leq</math> NMINT) is the number of modes for which the shape and stress values are desired.</p> <p>NOINTP is a limiter for the number of interpolations to be performed in refining the eigenfrequency value (<math>=10</math>).</p> <p>G is the gravitational constant.</p> <p>BMLGTH is the total beam length, to be checked against SSUM, the sum of the NSS subspans.</p> <p>NPAR(I), I=1,7 are the numbers of the different values of the seven parameters PAR(I,J) described above. Thus there are NPAR(1)=4 different values of point weight, etc.</p> <p>SS(I), I=1,NSS are the lengths of the subdivisions shown in Figure 7.</p> <p>Line 120 will contain a minimum of 3 items up to an indeterminate maximum, depending on the program length limitations, as follows:</p> <p>NSUPP(I), I=1,NOSUP, are integer representations of the support conditions to be investigated (refer to Figure 8).</p> <p>MSHP(I), I=1,NMSHP are the modes for which the deflection and stress values are desired, in addition to eigenfrequency. (If no eigenmodes are desired, then MSHP(1=NMSHP)=0).</p> <p>The loading, material and geometrical properties as shown in Figure 7 appear in single-column format together with the corresponding value of IPAR(I,J), which represents the number of the subdivision at which the property value occurs. Thus, the 3 lb. point load (<math>=</math>PAR(1,2)) appears at subdivision 7 (<math>=</math>IPAR(1,2)).</p>
105-115	10-11	
120	14-16	
125-250	23-27	

To verify that all input values have been read in from EMB10R properly, lines 6, 12, 17, 21 and 26 of EMB10 create the file MB10W1 as an echo check (Figure 12). EMB10 will be discussed with reference to the flow chart shown in Figure 11, which illustrates only the more involved steps in detail.

An error message (statement 153) will be printed out and operations will be terminated if SSUM, the total of  $SS(I), I=1, NSS$  is not equal to the value of BMLGTH as contained in EMB10R. This will afford an extra check on the values of  $SS(I)$  as stored in core during execution.

Statement 3 determines if a point load is present at the left-hand support, i.e. if  $IPAR(1,1)=1$ . If not, statements 3-6 add a zero mass at this point and shift the subscript  $J$  of  $PAR(I,J)$  and  $IPAR(I,J)$  up by unity. Lines 62-66 similarly detect a zero load at the right-hand support, necessitating  $PAR(1,NPAR1)=0$ .

The DO loops involving lines 67-85 distribute the continuous properties  $PAR(I,J), I=2,7$  over the whole beam, giving NSS values of each property (refer to line 69). If, say, two or more different values of uniformly distributed load exist, the YES branch of line 70 is taken in order to assign the appropriate value of  $J$  such that  $PAR(2,J)$  is repositioned at the correct subspan. Thus, for the existing problem,  $PAR(2,4)(=3 \text{ lb/in})$  as read in must become  $PAR(2,10)$  and so forth. Subsequently these redistributed values must be spread out over the intermediate subspans, e.g.  $PAR(2,10)$  will be applied to subspans 11-14. This is accomplished by lines 77-85. Referring to file MB10W2 as shown in Figure 13, the property values as redistributed can be seen; also note that 17 ( $=NSS$ ) values exist for each continuous property ( $I=2,7$ ).

In order to establish an initial guess, to commence eigenfrequency computation in EMB15, an extreme range of  $\omega_n$  is found in line 102. The lower bound,  $Y$ , is based on minimum stiffness and maximum distributed loading taken as continuous over the entire span and using a guided-free constraint. The upper bound,  $X$ , is based on maximum stiffness and minimum distributed loading, using

the clamped-clamped constraint. These values are entered in file MB10W3 as shown in Figure 14. Their natural logarithms are designated as FINCR, i.e. possible values for the initial increment. The employment of these values is completely discretionary.

EMB10 13:53 FRI. 09/05/69.

```
1 $FILE EMB10R,MB10W1,MB10W2,MB10W3
2 DIMENSION NPAR(7),PAR(7,20),IPAR(7,20),PARMAX(6),PARMIN(6),MSHP(20),
3 + ITEMP(20),PTEMP(20),SS(17),MINT(20),NSUPP(10)
4 READ (1,150) NSS,NOSUP,NMINT,NMSHP,NØINTP,G,BMLGTH,(NPAR(I),I=1,7)
5 150 FØRMAT(4X,5I5,2F9.3,7I3)
6 WRITE (2,160) NSS,NOSUP,NMINT,NMSHP,NØINTP,G,BMLGTH,(NPAR(I),I=1,7)
7 160 FØRMAT (29H      ECHØ CHECK ØF INPUT DATA/6X,4HNSS=,13,3X,
8 + 6HNØSUP=,13,3X,6HNMINT=,13,3X,6HNMSHP=,13,3X,7HNØINTP=,13/6X,
9 + 2HG=,F8.3,3X,7HØBMLGTH=,F8.3,13H  NPAR VALUES,7I3)
10 READ (1,151) (SS(I),I=1,NSS)
11 151 FØRMAT(4X,6F10.3)
12 WRITE (2,163) (SS(I),I=1,NSS)
13 163 FØRMAT(/6X,9HSS VALUES/(6X,6F10.3))
14 READ(1,152) (NSUPP(I),I=1,NØSUP),(MINT(I),I=1,NMINT),
15 + (MSHP(I),I=1,NMSHP)
16 152 FØRMAT(4X,16I4)
17 WRITE (2,164) (NØSUP,(NSUPP(I),I=1,NØSUP)),
18 + (NMINT,(MINT(I),I=1,NMINT)),(NMSHP,(MSHP(I),I=1,NMSHP))
19 164 FØRMAT (/6X,12HNSUPP VALUES/6X,*I4//6X,11HMINT VALUES
20 + /6X,*I4//6X,11HMSHP VALUES/6X,*I4)
21 WRITE (2,168)
22 168 FØRMAT(/6X,19HPAR AND IPAR VALUES)
23 DØ 200 I=1,7
24 NPR=NPAR(I)
25 READ (1,154) (PAR(I,J),IPAR(I,J),J=1,NPR)
26 WRITE (2,165) (PAR(I,J),IPAR(I,J),J=1,NPR)
27 200 CØNTINUE
28 154 FØRMAT(4X,E11.4,I3)
29 165 FØRMAT(4(E11.4,I3)))
30 ENDFILE 2
31 WSUM=0.
32 NPAR1=NPAR(1)
33 DØ 121 I=1,NPAR1
34 121 WSUM=WSUM+PAR(1,I)
35 DØ 122 I=2,6
36 NPARX=NPAR(I)
37 PARMAX(I)=PAR(I,1)
38 PARMIN(I)=PAR(I,1)
39 IF (IPAR(I,NPARX)-1) 12,122,12
40 12 DØ 122 J=2,NPARX
41 PARMIN(I)=AMIN1(PARMIN(I),PAR(I,J))
42 122 PARMAX(I)=AMAX1(PARMAX(I),PAR(I,J))
43 SSUM=0.
44 DØ 1 I=1,NSS
45 1 SSUM=SSUM+SS(I)
46 IF(ABS(BMLGTH-SSUM)-.0001)3,3,2
47 2 PRINT 153,SSUM
48 153 FØRMAT (54H INEQUALITY ØF BMLGTH AND CØPUTED BEAMLENGTH,
49 + LATTER=,F10.3)
50 GØTØ 99
51 3 IF(IPAR(1,1)-1)4,7,4
52 4 NPAR(1)=NPAR(1)+1
53 NPAR1=NPAR(1)
```

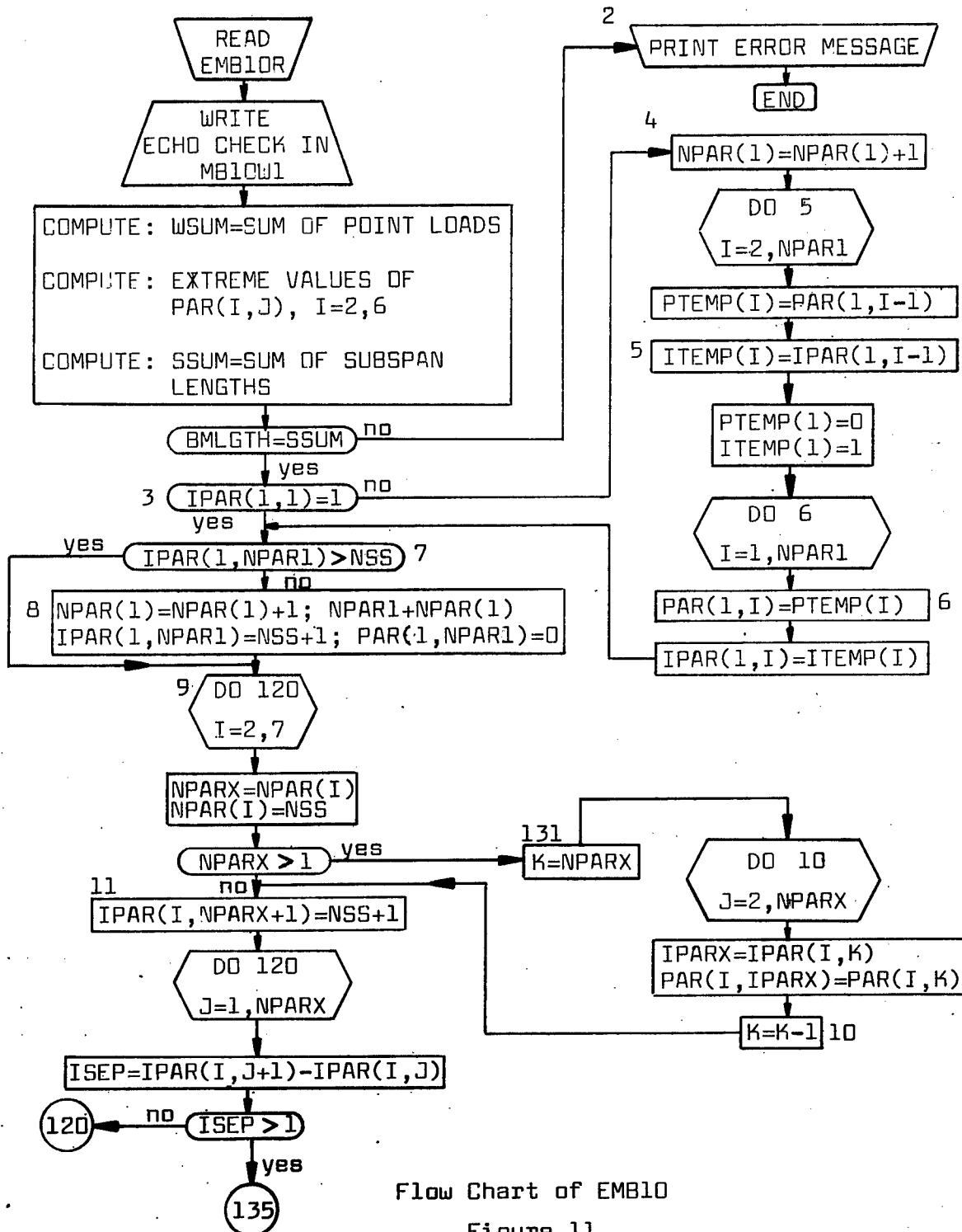
Program MEMB10

Figure 10

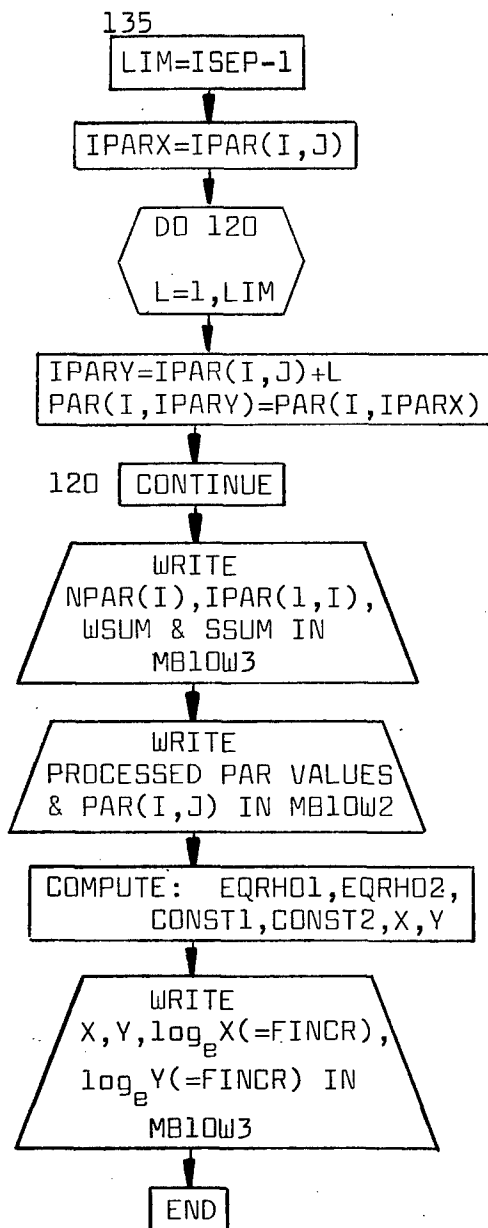
```

54 DO 5 I=2,NPAR1
55 PTEMP(1)=PAR(1,I-1)
56 5 ITEMP(1)=IPAR(1,I-1)
57 PTEMP(1)=0.
58 ITEMP(1)=1
59 DO 6 I=1,NPAR1
60 PAR(1,I)=PTEMP(1)
61 6 IPAR(1,I)=ITEMP(1)
62 7 IF(IPAR(1,NPAR1)-NSS)8,8,9
63 8 NPAR(1)=NPAR(1)+1
64 NPAR1=NPAR(1)
65 IPAR(1,NPAR1)=NSS+1
66 PAR(1,NPAR1)=0.
67 9 DO 120 I=2,7
68 NPARX=NPAR(1)
69 NPAR(1)=NSS
70 IF (NPARX-1) 11,11,131
71 131 K=NPARX
72 DO 10 J=2,NPARX
73 IPARX=IPAR(1,K)
74 PAR(1,IPARX)=PAR(1,K)
75 10 K=K-1
76 11 IPAR(1,NPARX+1)=NSS+1
77 DO 120 J=1,NPARX
78 ISEP=IPAR(1,J+1)-IPAR(1,J)
79 IF(ISEP-1)120,120,135
80 135 LIM=ISEP-1
81 IPARX=IPAR(1,J)
82 DO 120 L=1,LIM
83 IPARY=IPAR(1,J)+L
84 PAR(1,IPARY)=PAR(1,IPARX)
85 120 CONTINUE
86 WRITE (4,161) (NPAR(1),I=1,7)
87 161 FORMAT (6X,11HNPAR VALUES, 7I3)
88 WRITE (4,156) (NPAR1,(IPAR(1,I),I=1,NPAR1))
89 156 FORMAT(/6X,11HIPAR VALUES/7X,*I4)
90 WRITE (4,157) WSUM,SSUM
91 157 FORMAT (/6X,5HWSUM=,E15.8,7H SSUM=,E15.8)
92 WRITE (3,169); 169 FORMAT(5X,20HPRØCESSED PAR VALUES)
93 DO 201 I=1,7
94 NPR=NPAR(1)
95 201 WRITE (3,155) (PAR(1,J),J=1,NPR)
96 155 FORMAT(6X,4E15.8)
97 ENDFILE 3
98 EQRH01=(WSUM/SSUM+PARMIN(2)+PARMIN(3)*PARMIN(6))/G
99 EQRH02=(WSUM/SSUM+PARMAX(2)+PARMAX(3)*PARMAX(6))/G
100 CONST1=(PARMAX(4)*PARMAX(5)/(EQRH01*SSUM**4))**.5
101 CONST2=(PARMIN(4)*PARMIN(5)/(EQRH02*SSUM**4))**.5
102 WRITE(4,167) X=4.73^2*CONST1,Y=1.571^2*CONST2,LOG(X),LOG(Y)
103 167 FORMAT(/54H EXTREME RANGE ØF 1ST/2ND MØDE CIRCULAR
104 + FREQUENCY/2E15.8/6HFINCR=,F7.4,11H ØR FINCR=,F7.4)
105 ENDFILE 4
106 99 CONTINUE
107 $OPT SIZE
108 END
LENGTH
ADOUT 3200 CHARS.

```







MB10W1 17:42 WED. 09/03/69.

ECHØ CHECK ØF INPUT DATA

NSS= 17 NØSUP= 3 NMINT= 5 NMSHP= 5 NØINTP= 10  
G= 386.000 BMLGTH= 36.000 NPAR VALUES 4 5 4 4 3 3 3

SS VALUES

3.400	1.700	2.100	2.400	1.400	.900
3.000	3.400	.800	2.700	2.100	2.100
1.100	3.100	1.900	1.700	2.200	

NSUPP VALUES

2 7 10

MINT VALUES

1 2 3 4 5

MSHP VALUES

1 2 3 4 5

PAR AND IPAR VALUES

.7000E+01	3	.3000E+01	7	.4000E+01	11	.6000E+01	16
.0000E+01	1	.2000E+01	2	.0000E+01	5	.3000E+01	10
.0000E+01	15						
.1000E+00	1	.3000E+00	4	.2000E+00	9	.5000E-01	17
.3000E+08	1	.3500E+08	6	.1000E+08	12	.3300E+08	14
.4500E-03	1	.5210E-02	8	.0125E-03	13		
.6000E-01	1	.2500E+00	8	.1500E-01	13		
.1500E+00	1	.2500E+00	8	.5000E-01	13		

Permanent File MB10W1

Figure 12

MB10W2 13:39 THUR. 03/14/69.

PROCESSED PAR VALUES

PAR(1,J)	.00000000E+01	.70000000E+01	.30000000E+01	.40000000E+01
	.60000000E+01	.00000000E+01		
	.00000000E+01	.20000000E+01	.20000000E+01	.20000000E+01
	.00000000E+01	.00000000E+01	.00000000E+01	.00000000E+01
PAR(2,J)	.00000000E+01	.30000000E+01	.30000000E+01	.30000000E+01
	.30000000E+01	.30000000E+01	.00000000E+01	.00000000E+01
	.00000000E+01			
	.10000000E+00	.10000000E+00	.10000000E+00	.30000000E+00
	.30000000E+00	.30000000E+00	.30000000E+00	.30000000E+00
PAR(3,J)	.20000000E+00	.20000000E+00	.20000000E+00	.20000000E+00
	.20000000E+00	.20000000E+00	.20000000E+00	.20000000E+00
	.50000000E-01			
	.30000000E+08	.30000000E+08	.30000000E+08	.30000000E+08
	.30000000E+08	.35000000E+08	.35000000E+08	.35000000E+08
PAR(4,J)	.35000000E+08	.35000000E+08	.35000000E+08	.10000000E+08
	.10000000E+08	.33000000E+08	.33000000E+08	.33000000E+08
	.33000000E+08			
	.45000000E-03	.45000000E-03	.45000000E-03	.45000000E-03
	.45000000E-03	.45000000E-03	.45000000E-03	.52100000E-02
PAR(5,J)	.52100000E-02	.52100000E-02	.52100000E-02	.52100000E-02
	.01250000E-03	.01250000E-03	.01250000E-03	.01250000E-03
	.01250000E-03			
	.60000000E-01	.60000000E-01	.60000000E-01	.60000000E-01
	.60000000E-01	.60000000E-01	.60000000E-01	.25000000E+00
PAR(6,J)	.25000000E+00	.25000000E+00	.25000000E+00	.25000000E+00
	.15000000E-01	.15000000E-01	.15000000E-01	.15000000E-01
	.15000000E-01			
	.15000000E+00	.15000000E+00	.15000000E+00	.15000000E+00
	.15000000E+00	.15000000E+00	.15000000E+00	.25000000E+00
PAR(7,J)	.25000000E+00	.25000000E+00	.25000000E+00	.25000000E+00
	.50000000E-01	.50000000E-01	.50000000E-01	.50000000E-01
	.50000000E-01			

Permanent File MB10W2

Figure 13

MB10W3 17:44 WED. 09/03/69.

NPAR VALUES 6 17 17 17 17 17 17

IPAR VALUES

1 3 7 11 16 18

WSUM= .20000000E+02 SSUM= .36000000E+02

EXTREME RANGE OF 1ST/2ND MODE CIRCULAR FREQUENCY

.19418117E+03 .21953789E+00

FINCR= 5.2688 OR FINCR=-1.5162

Permanent File MB10W3

Figure 14

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### Computation of Eigenfrequencies and State Vectors

The saved items EMB15/20/21 accept the data conditioned by EMB10 and compute the eigenfrequencies and state vectors for each MINT(I) and NSUPP(II) without interruption until the problem is completed. If eigenmodes or further refinement of particular eigenfrequencies are desired, however, execution may be terminated. For efficiency, solutions may begin at any value of guessed frequency and any mode at the discretion of the user. Array storage requirements have been minimized by this restart feature; thus when 30 values of frequency  $FREQ(I)$  have been computed and used to determine the corresponding frequency function  $DIF(I)$  and more are desired, execution is recommenced by the user with the index I reset to 1 and employing the previous frequency as  $FREQ(1)$ . If the current  $MINT=MSHP(IN)$ , i.e. the eigenmode is desired, the required data corresponding to the eigenfrequency as computed after ten interpolations are entered into file MB15W1 and linked file MB15W2/MB15W3 by EMB20 and the message "RUN EMB25 FOR MODAL VALUES" is printed.

The flow chart of EMB15 is shown in Figure 16 and the program itself appears as Figure 15. The input is largely from permanent files EMB10R, MB10W2 and MB10W3. Terminal-supplied input (lines 48-50) is confined to values which implement the restart feature.

<u>Files</u>	<u>Read by EMB15 Lines</u>	<u>EXPLANATION</u>
EMB10R	10-22	Refer to "Conditioning of Input Data".
MB10W2	30-38	Refer to Figure 13.
MB10W3	24-26	This input furnishes the number of point loads to be considered, including virtual loads, if any. (NPAR(I), I=2,7 are all =NSS)

The subspans corresponding to each point load,  $IPAR(1,J)$  are needed for the computation of point transfer matrices.

Initially and when reentry into the program is attempted a message is printed requesting seven parameters from the terminal:

- 1) II is the index of the support condition NSUPP and can be changed at will to allow, for example, an investigation for eigenvalues within a given frequency range for various end constraints.
- 2) IM & IN are the subscripts of MINT and MSHP respectively and can be set independently to allow the determination of an eigenfrequency of interest for other than MINT(1).
- 3) NMODE denotes the mode currently being investigated.
- 4) FREQ(1) is the initial frequency to be used as a starting point in the spectrum under examination.
- 5) FINCR is the natural logarithmic increment of frequency to be added to FREQ(1) and may be either arbitrary or drawn from the extreme range of values as computed by EMB10. Negative values must be avoided.
- 6) IND(I) indicates the polarity of the frequency function  $DIF(I)$  ( $=R(\omega)$ ); i.e.  $IND(I)=1$  if  $DIF(I)>0$ , otherwise  $IND(I)=2$ .  $IND(I)$  is needed to initialize the procedure in EMB20 for detecting polarity reversal, i.e. if  $(IND(I)-IND(I-1))$ . The user will initially not know the sign of  $DIF(1)$  and thus a restart may be indicated if the wrong choice is made.

In line 42 an arithmetic statement function is defined which represents the procedure for interpolating between the previous frequency,  $FREQ(I-1)$  and  $FREQ(ICROSS)$ , the last frequency corresponding to a  $DIF$  value of different sign.

In lines 52-54 the frequency-independent  $B(J)$ ,  $J=1,NSS$  are computed for use later in establishing field transfer matrices. Lines 58-68 set  $FRQN(1)=0$  for the first four support conditions and increment  $IM$  and  $IN$  to 2 (if  $MINT(1)=1$  through oversight). The  $SS(J)$  values have not been included in  $B(J)$  as an expedient for

saved item EMB25, which reads the COEFM(J) and further subdivides the subspans; thus it is desirable that  $T (=COEFM(J)=B(J)*FREQ(I)**\frac{1}{2})$  does not involve SS(J) at this stage. If, at line 56,  $NMODE \geq 2$ , the immediately preceding eigenfrequency will be supplied at the terminal. This is utilized by EMB20 to compute a FINCR for the ensuing mode, i.e. for  $NMODE+1$ .

In line 72,  $FREQ(2)$  is determined exponentially; arithmetic incrementing would be less efficient in the preliminary detection of eigenvalues. Care should be taken not to supply  $FRQN(1)=0$  to avoid an execution error in connection with the intrinsic LOG function:

Statement 16 is a computed GOTO which initializes the first state column vector STAMAT and correction column vector CORMAT according to the support condition. Thus for the guided-free constraint ( $NSUPP(1)=2$ ) the initial vectors are:

$$\begin{aligned} STAMAT_1 &= \{1 \ 1 \ 0 \ 0\} \\ CORMAT_1 &= \{0 \ 1 \ 0 \ 0\} \end{aligned} \quad (16)$$

Here the free-end deflection is normalized to one inch and the slope chosen as unity. Lines 84-128 establish the transfer matrices, indices K and J being incremented at each point load and elastic field respectively. Figure 24 illustrates how the K and J values are assigned for this beam. Whenever  $J=IPAR(1,K)$  a point transfer matrix is computed in lines 88-90. If, subsequently,  $NSS < IPAR(1,K)$ , (line 92) then matrix computation is completed for  $FREQ(I)$  and control passes to EMB21 to obtain the state vectors and correction factor by chain multiplication; otherwise computation of the Jth field matrix is indicated—according to Figure 3 if the field is effectively massless (i.e. if  $PAR(3,J) \leq .0001$ ) or Figure 2 otherwise. The argument of the natural trigonometric and hyperbolic functions assumes its largest value at  $SS(13)$ , i.e.  $ARG_{max} = .097724 \omega^{\frac{1}{2}}$  and the largest permissible argument =  $|176|$ , which corresponds to  $\omega_{max} = 517$  kHz approximately. Argument magnitude difficulties may be present with beams having longer and more compliant elastic fields. In such cases a virtual load could be inserted at midspan, thus halving the argument.

For example, the natural frequencies in pure bending of a 1" square uniform aluminum beam 25" long and clamped at each end, as reported in Reference 2, were corroborated to nearly 27 000 rad/sec (until the argument exceeded [176] ).

EMB21 appears as Figure 17 and is charted in Figure 18. Its function, as an item which is compiled with EMB20 and EMB25, is to obtain the intermediate and final state vectors (STAMAT(J,K,1), J=2,NSS+NPARG(1)+1), K=1,4 and the elements of the correction vector CORMAT(J,K,1). At each FREQ(I) as mentioned previously two iterations are needed to obtain DIF(I); ITER=0 causes both vectors to be computed in lines 154-160. If ITER=1 only STAMAT is recomputed. By the computed GO TO in line 162 the components COR1 and COR2 of the correction term COR are obtained in accordance with the support condition. Thus for this beam slope and shear force at the right-hand support are ideally =0, causing M=2 and N=4. COR1 and COR2 are obtained from the relationship

$$\text{STAMAT}(\text{LIM}+1,2,1) + \text{COR1} * \text{CORMAT}(\text{LIM}+1,2,1) = 0$$

$$= \Psi_{m+n+1} \quad (17)$$

$$\text{STAMAT}(\text{LIM}+1,4,1) + \text{COR2} * \text{CORMAT}(\text{LIM}+1,4,1) = 0$$

$$= V_{m+n+1} \quad (18)$$

The correction term COR is taken as the arithmetic mean of COR1 and COR2 and is applied in statement 105 to the left-hand slope, the other elements of  $Z_1$  remaining the same. ITER is now made =1 and statement 300 is reentered. After the recomputation of STAMAT, exit is made to statement 119 in either EMB20 or EMB25 as appropriate. In EMB20 (refer to Figures 19 and 20) statement 119 computes the frequency function as the difference between the components of  $Z_{m+n+1}$  (which should be zero at the eigenfrequency). According to {1} this difference is to be taken as COR1-COR2, but this involves magnitudes that become progressively smaller as NMODE increases. This results from the growth of the CORMAT elements with NMODE, causing the graph of DIF vs. FREQ to flatten out. In some cases this may hinder eigenfrequency determination if sufficiently severe.



Statement 80 causes control to branch to 84 if interpolation is not being performed, i.e. if the frequency axis has not yet been crossed for the current  $NMODE=MINT(IM)$ . The test for polarity reversal of  $DIF(I)$  is statement 84—the YES branch being taken to obtain additional  $DIF$  values until a reversal does occur, whereupon the current  $I$  is retained in  $MAX$  to preserve  $IND(I)$  and also allow frequency incrementing to commence with  $FREQ(MAX)$  after all interpolations are completed. The first approximation to  $FRQN(NMODE)$  is a linear interpolation between  $FREQ(I)$  and  $FREQ(I-1)$  and is printed by lines 21 and 23. After this first axis crossing, if  $NMODE=MINT(IM)$  the YES branch is taken to 97 and ten ( $=NOINTP$ ) further interpolations are performed to refine  $FRQN$ .  $I-1$  is retained in  $ICROSS$  to preserve the  $FREQ$  value previous to the axis crossing. The first approximation to  $FRQN(NMODE)$  is put into  $FREQ(I+1)$  and control is returned to statement 16 in  $EMB15$  to obtain the corresponding  $DIF$ . Interpolation proceeds until  $INTERP \geq NOINTP$ , at which point the final value of  $FRQN$  is equated to the current  $FREQ$ . If a sign change in  $DIF$  occurs during interpolation and  $INTERP < NOINTP$ , then lines 37 and 39 assign the current  $I-1$  to  $ICROSS$  and control is returned to  $EMB15$ .

If the current eigenfrequency was the last to be determined, i.e.  $NMODE=MINT(NMINT)$ , then statement 133 determines if the eigenmode is also required, i.e. if  $NMODE=MSHP(NMSHP)$ . The NO branch from 133 leads to the question of whether or not all support conditions have been investigated, i.e. if  $NOSUP > II$ . Here the NO branch indicates problem completion, a notice to this effect being printed by statement 134; otherwise 139 causes a request to be printed for the new  $NSUPP$ , and control is returned to input statement 12 of  $EMB15$ . The YES branch from 133 causes all data needed by  $EMB25$  to be entered into permanent files  $MB15W1$  and  $MB15W2/MB15W3$  by lines 79-99. This involves the state vectors  $STAMAT$  and all transfer matrices  $BMAT$ ; format statements 75, 76, 78 and 79 arrange the elements to facilitate visualization of the matrix multiplication (refer to Figure 21 which shows the first five state vectors for the fourth eigenfrequency of 156.7256 rad/sec ( $=24.944$  Hz)).  $MB15W1$  is shown

in Figure 22.  $FRQN(NMODE)$  is added for completeness and  $FINCR$  will be required when EMB15 is rerun.

Lines 59-77 establish the new increment for the succeeding  $NMODE$  on the basis of support condition and the value of  $NMODE$ . Thus for  $NMODE=1$  or  $2$  and  $NATSP=1,2,3$  or  $4$  there exists no  $FRQN(NMODE-1)$  as required by statement 72, causing  $FINCR=.1$ . Otherwise statement 72 assigns a value to  $FINCR$  based on the log ratio of the current to the previous  $FRQN$  if  $NMODE=2$ ; if  $NMODE=1$  for  $NATSP=5-10$  then  $FINCR=.1$ . After  $FINCR$  has been determined by statement 70 or 72 it is recorded in MB15W1 if the eigenmode is desired, otherwise line 73 increments  $I$ ,  $IM$  and  $NMODE$ . The  $IND(I-1)$  and  $FREQ(I)$  values are based on  $IND(MAX)$  and  $FREQ(MAX)$ .

Figure 23 shows the results for  $NMODE=NATSP=2$ . The sensitivity of  $DIF$  to  $FREQ$  is quite apparent at  $I=19$  and  $20$ , where 9th and 10th place differences in  $FREQ$  cause an 80.2% decrease in  $DIF$ .

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```

2 $FILE EMB10R,MB10W2,MB10W3,MB15W1,MB15W2/MB15W3
4 DIMENSION BMAT(23,4,4),COEFM(17),FREQ(30),DIF(30),IND(30),
6 + FRQN(30),MINT(30),MSHP(30),NSUPP(10),NPAR(7),SS(17),
8 + PAR(7,20),IPAR(1,20),B(17),STAMAT(24,4,1),CORMAT(24,4,1)
10 READ (1,290) NSS,NOSUP,NMINT,NMSHP,NINIP,G
12 290 FORMAT (4X,5I5,F9.3)
14 READ (1,292) (SS(I),I=1,NSS)
16 292 FORMAT (4X,6F10.3)
18 READ (1,291) (NSUPP(I),I=1,NOSUP),(MINT(I),I=1,NMINT),
20 + (MSHP(I),I=1,NMSHP)
22 291 FORMAT (4X,16I4)
24 READ(3,293) (NPAR(I),I=1,7)
26 293 FORMAT (17X,7I3)
28 READ(2)
30 DO 294 I=1,7
32 NPR=NPAR(I)
34 NPAR1=NPAR(I)
36 294 READ (2,295) (PAR(I,J),J=1,NPR)
38 295 FORMAT (6X,4E15.8)
40 READ(3,297) (NPAR1,(IPAR(1,I),I=1,NPAR1))
42 297 FORMAT(/7X,*I4); R(I)=-ABS(DIF(I-1))/(ABS(DIF(I-1))
44 + +ABS(DIF(ICROSS)))*(FREQ(I-1)-FREQ(ICROSS))+FREQ(I-1)
46 LIM=NSS+NPAR(1); 12 PRINT,"INPUT VALUES OF II,IM,IN,NMODE,FREQ(1),
48 +FINCR AND IND(1)"; INPUT,II,IM,IN,NMODE,FREQ(1),FINCR,IND(1)
50 NATSP=NSUPP(II); ITER=INTERP=0; I=1
52 DO 11 J=1,NSS; 11 B(J)=((PAR(2,J)+PAR(3,J)*PAR(6,J))/
54 + (G*PAR(4,J)*PAR(5,J)))*.25
56 IF(NMODE-1) 201,201,202; 202 PRINT 203,"FRQN(",NMODE-1,")="
58 203 FORMAT(I2); INPUT, FRQN(NMODE-1); GO TO 15
60 201 GO TO(13,13,13,13,15,15,15,15,15,15), NATSP
62 13 FRQN(1)=0.
64 NMODE=2
66 IF(MINT(1)-1) 15,200,15
68 200 IM=IN=2
70 15 I=I+1
72 FREQ(I)=EXP(LOG(FREQ(I-1))+FINCR)
74 16 GO TO (25,25,25,26,26,28,29,28,28,28), NATSP
76 25 STAMAT(1,1,1)=STAMAT(1,2,1)=1.; GO TO 27
78 26 STAMAT(1,1,1)=STAMAT(1,3,1)=1.; GO TO 32
80 28 STAMAT(1,3,1)=STAMAT(1,4,1)=1.; GO TO 32
82 29 STAMAT(1,2,1)=STAMAT(1,4,1)=1.; 27 CORMAT(1,2,1)=1.; GO TO 39
84 32 CORMAT(1,3,1)=1.; 39 K=1; J=0; 17 J=J+1
86 IF(J-IPAR(1,K)) 23,20,23
88 20 DO 22 L=1,4; 22 BMAT(J+K-1,L,L)=1.; BMAT(J+K-1,4,1)=PAR(1,K)
90 + *FREQ(I)**2/G
92 IF(NSS-IPAR(1,K)) 24,19,19; 19 K=K+1
94 23 N=J+K-1; EI=PAR(4,J)*PAR(5,J); IF(PAR(3,J)-.0001) 5,5,6
96 5 COEFM(J)=0.; DO 2 L=1,4; DO 1 M=1,4; 1 BMAT(N,L,M)=0.
98 2 BMAT(N,L,L)=1.; BMAT(N,1,2)=BMAT(N,3,4)=SS(J)
100 BMAT(N,1,3)=BMAT(N,2,4)=SS(J)**2/(2.*EI); BMAT(N,2,3)=SS(J)/EI
102 BMAT(N,1,4)=SS(J)**3/(6.*EI); GO TO 17

```

Saved Item EMB15

Figure 15

```

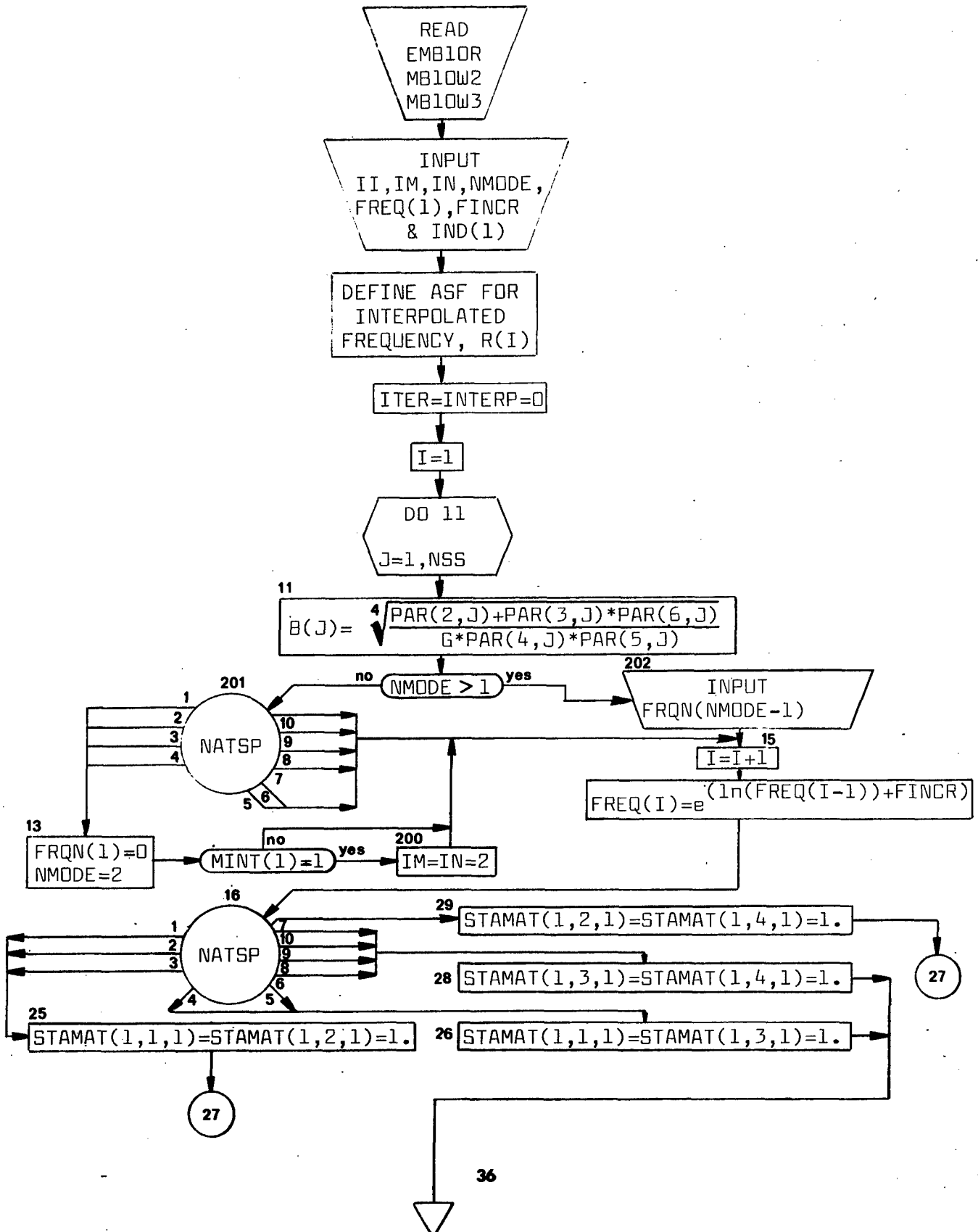
104 6 T=COEFM(J)=B(J)*FREQ(I)**.5; ARG=T*SS(J)
106 U=(EXP(ARG)+EXP(-ARG))/2.; V=COS(ARG); W=U-V
108 X=(EXP(ARG)-EXP(-ARG))/2.; Y=SIN(ARG); Z=X-Y
110 BMAT(N,1,1)=BMAT(N,2,2)=BMAT(N,3,3)=BMAT(N,4,4)=(U+V)/2.
112 BMAT(N,1,2)=BMAT(N,3,4)=(X+Y)/(2.*T)
114 BMAT(N,1,3)=BMAT(N,2,4)=W/(2.*EI*T**2)
116 BMAT(N,1,4)=Z/(2.*EI*T**3)
118 BMAT(N,2,1)=BMAT(N,4,3)=T*Z/2.
120 BMAT(N,2,3)=(X+Y)/(2.*EI*T)
122 BMAT(N,3,1)=BMAT(N,4,2)=EI*T**2*Z/2.
124 BMAT(N,3,2)=EI*T*Z/2.
126 BMAT(N,4,1)=EI*T**3*(X+Y)/2.
128 GO TO 17
130 24 $USE EMB20

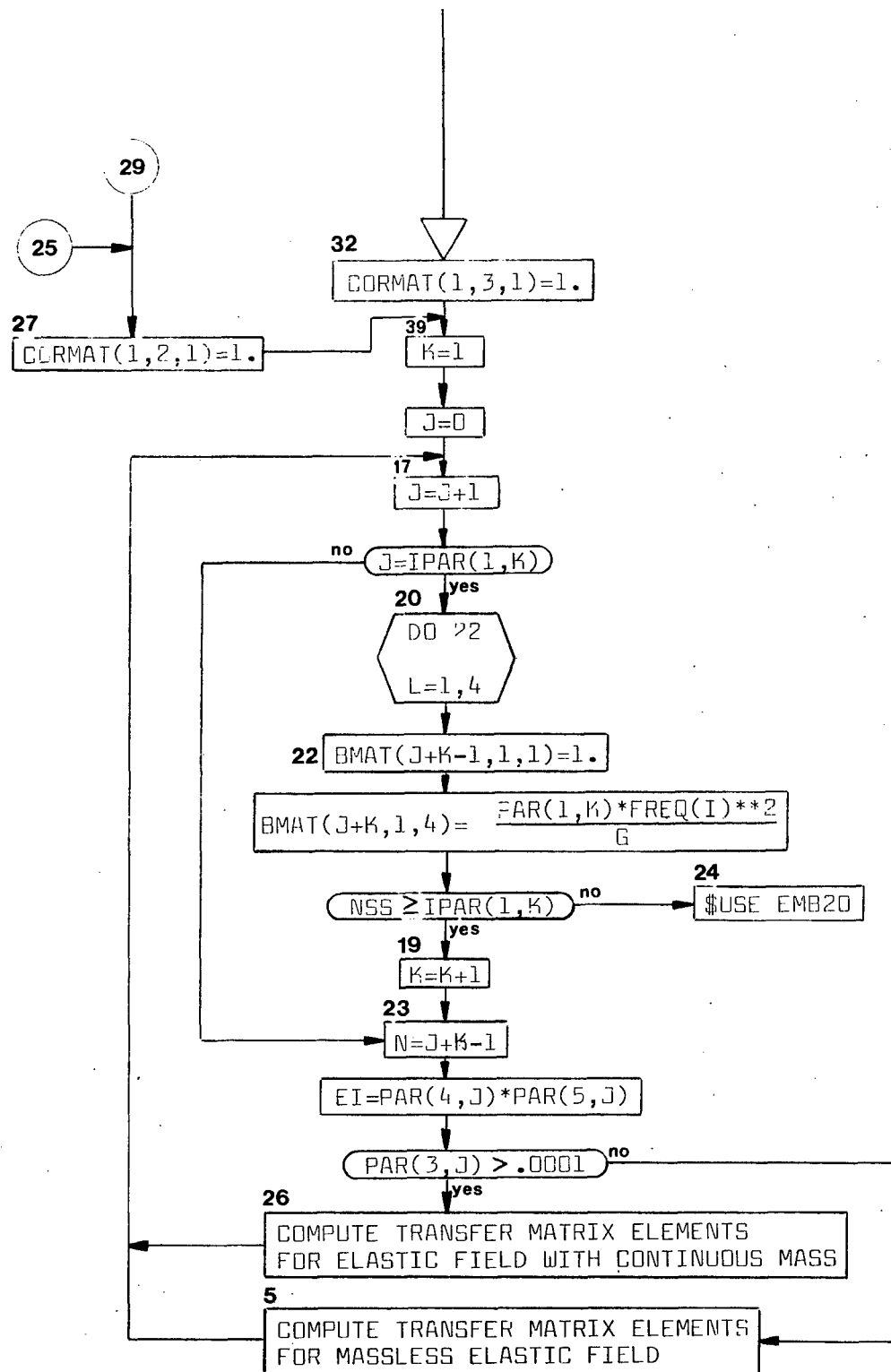
```

LENGH  
 ABOUT 3000 CHARS.

# Flow Chart of EMB15

Figure 16





EMB21 4:38 SAT. 09/06/69.

```
150 300 DØ 310 J=1,LIM; DØ 310 M=1,4; STAMAT(J+1,M,1)=0.
152 IF(ITER) 310,180,310; 180 CØRMAT(J+1,M,1)=0.; 310 CØNTINUE
154 DØ 340 J=1,LIM; DØ 340 M=1,4; DØ 340 N=1,4
156 STAMAT(J+1,M,1)=STAMAT(J+1,M,1)+STAMAT(J,N,1)*BMAT(J,M,N)
158 IF(ITER) 340,350,340; 350 CØRMAT(J+1,M,1)=CØRMAT(J+1,M,1)+
160 + CØRMAT(J,N,1)*BMAT(J,M,N); 340 CØNTINUE
162 GØ TØ (113,114,115,114,115,113,115,115,114,116), NATSP
164 113 M=3; N=4; GØ TØ 118
166 114 M=2; N=4; GØ TØ 118
168 115 M=1; N=3; GØ TØ 118
170 116 M=1; N=2; 118 IF(ITER) 119,121,119; 121 CØR1=
172 + -STAMAT(LIM+1,M,1)/CØRMAT(LIM+1,M,1)
174 CØR2=-STAMAT(LIM+1,N,1)/CØRMAT(LIM+1,N,1)
176 CØR=(CØR1+CØR2)/2.
178 GØ TØ (105,105,105,106,106,106,105,106,106,106), NATSP
180 105 STAMAT(1,2,1)=STAMAT(1,2,1)+CØR; GØ TØ 330
182 106 STAMAT(1,3,1)=STAMAT(1,3,1)+CØR; 330 ITER=1; GØ TØ 300
```

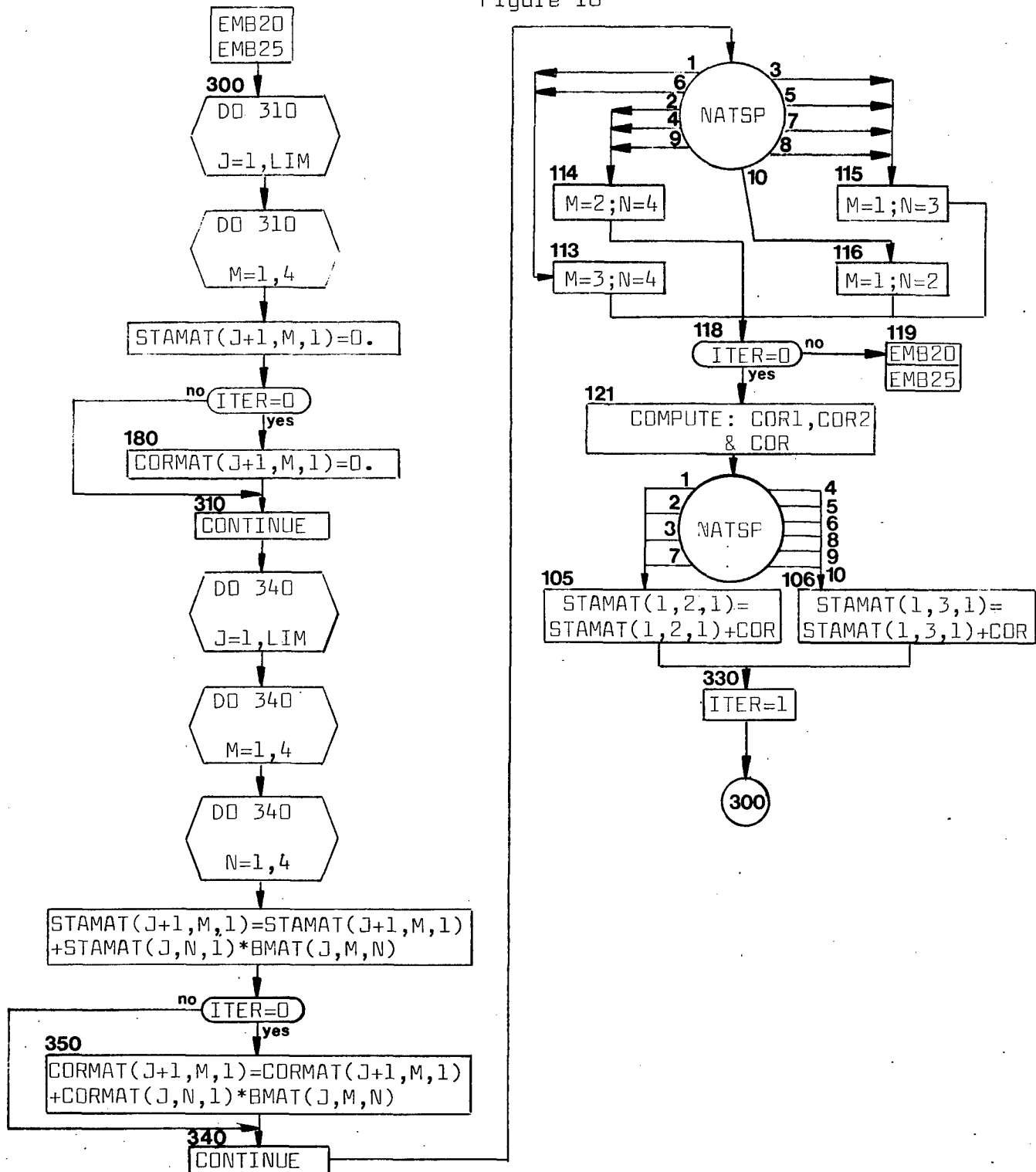
Saved Item EMB21

Figure 17



# Flow Chart of EMB21

Figure 18



EMB20 18:02 MØN. 08/18/69.

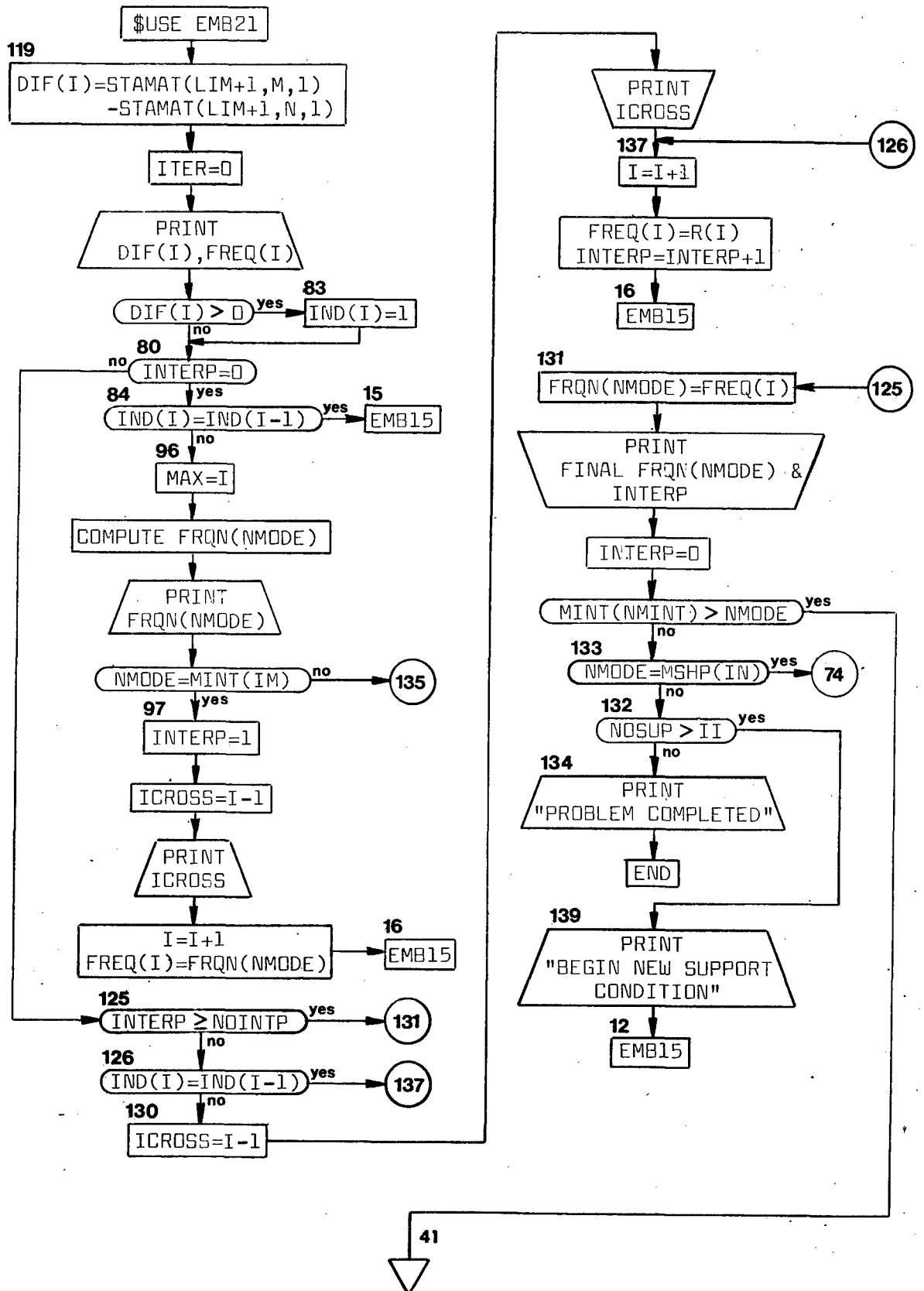
```
1 $USE EMB21
3 119 DIF(I)=STAMAT(LIM+1,M,1)-STAMAT(LIM+1,N,1); ITER=0
5 PRINT 401, "DIFFERENCE VALUE(",I,")=",DIF(I), " FREQ=",FREQ(I)
7 401 FØRMAT(I2,2E15.8)
9 IF(DIF(I)) 81,81,83
11 81 IND(I)=2; GØ TØ 80; 83 IND(I)=1
13 80 IF(INTERP) 125,84,125
15 84 IF(IND(I-1)-IND(I)) 96,15,96
17 96 MAX=I; FRQN(NMØDE)=ABS(DIF(I-1))/(ABS(DIF(I))+ABS(DIF(I-1)))
19 + *(FREQ(I)-FREQ(I-1))+FREQ(I-1)
21 PRINT 402, "ROUGH FRQN(",NMØDE,")=",FRQN(NMØDE), "= ",
23 + FRQN(NMØDE)/6.2832, "HZ", "^"; 402 FØRMAT(I2,2E15.8)
25 IF(NMØDE-MINT(IM)) 135,97,135
27 97 INTERP=1; ICRØSS=I-1; PRINT 403, ICRØSS; I=I+1
29 FREQ(I)=FRQN(NMØDE); GØ TØ 16
31 403 FØRMAT(7HICRØSS=,I4/)
33 125 IF(NØINTP-INTERP) 131,131,126
35 126 IF(IND(I)-IND(I-1)) 130,137,130
37 130 ICRØSS=I-1; PRINT 403,ICRØSS; 137 I=I+1; FREQ(I)=R(I)
39 INTERP=INTERP+1; GØ TØ 16
43 131 FRQN(NMØDE)=FREQ(I)
45 PRINT 405, "FINAL FRQN(",NMØDE,")=",FRQN(NMØDE), "= ",FRQN(NMØDE)
47 + /6.2832, "HZ", " INTERP=",INTERP; 405 FØRMAT(I2,2E15.8,I3//)
49 INTERP=0
51 IF(MINT(NMINT)-NMØDE) 133,133,135; 133 IF(NMØDE-MSHP(IN))132,74,132
53 132 IF(NØSUP-II) 134,134,139; 139 PRINT, "BEGIN NEW SUPPØRT
55 + CØNDITION"; GØ TØ 12
57 134 PRINT, "PRØBLEM CØMPLETED"; GØ TØ 304
59 135 IF(NMØDE-2) 136,136,72
61 136 GØ TØ (70,70,70,70,71,71,71,71,71,71), NATSP
63 70 FINCR=.1
65 IF(NMØDE-MSHP(IN)) 73,74,73
67 71 GØ TØ (70,72),NMØDE
69 72 FINCR=.1*LØG(FRQN(NMØDE)/FRQN(NMØDE-1))
71 IF(NMØDE-MSHP(IN)) 73,74,73
73 73 I=I+1; IM=IM+1; IND(I-1)=IND(MAX); NMØDE=NMØDE+1
75 FREQ(I)=EXP(LØG(FREQ(MAX))+FINCR)
77 GØ TØ 16
79 74 WRITE (5,75); 75 FØRMAT(59X,9HSTAMAT(1))
81 WRITE (5,76) (STAMAT(1,M,1),M=1,4); 76 FØRMAT (57X,E14.8)
83 DØ 77 J=1,LIM; WRITE (5,78) J,J+1
85 78 FØRMAT(/5HBMAT(I2,1H),51X,7HSTAMAT(I2,1H))
87 DØ 77 K=1,4; WRITE (5,79) (BMAT(J,K,L),L=1,4),STAMAT(J+1,K,1)
89 79 FØRMAT(4E14.8,X,E14.8); 77 CØNTINUE; ENDFILE 6
91 WRITE(4,69) NMØDE,MINT(NMINT),II,NATSP,FRQN(NMØDE),
93 + (CØRMAT(1,M,1),M=1,4),(J,CØEFM(J),J=1,NSS)
95 69 FØRMAT(6HNMØDE=,I2,14H MINT(NMINT)=,I2,5H II=,I2,
97 + 8H NATSP=,I2,7H FRQN=,E15.8/9HCØRMAT(1),4F4.1
99 + /(6HCØEFM(I2,3H)=,E15.8)); ENDFILE 4
101 PRINT, "RUN EMB25 FØR MØDAL VALUES"
103 304 CØNTINUE
105 $ØPT SIZE
107 END
```

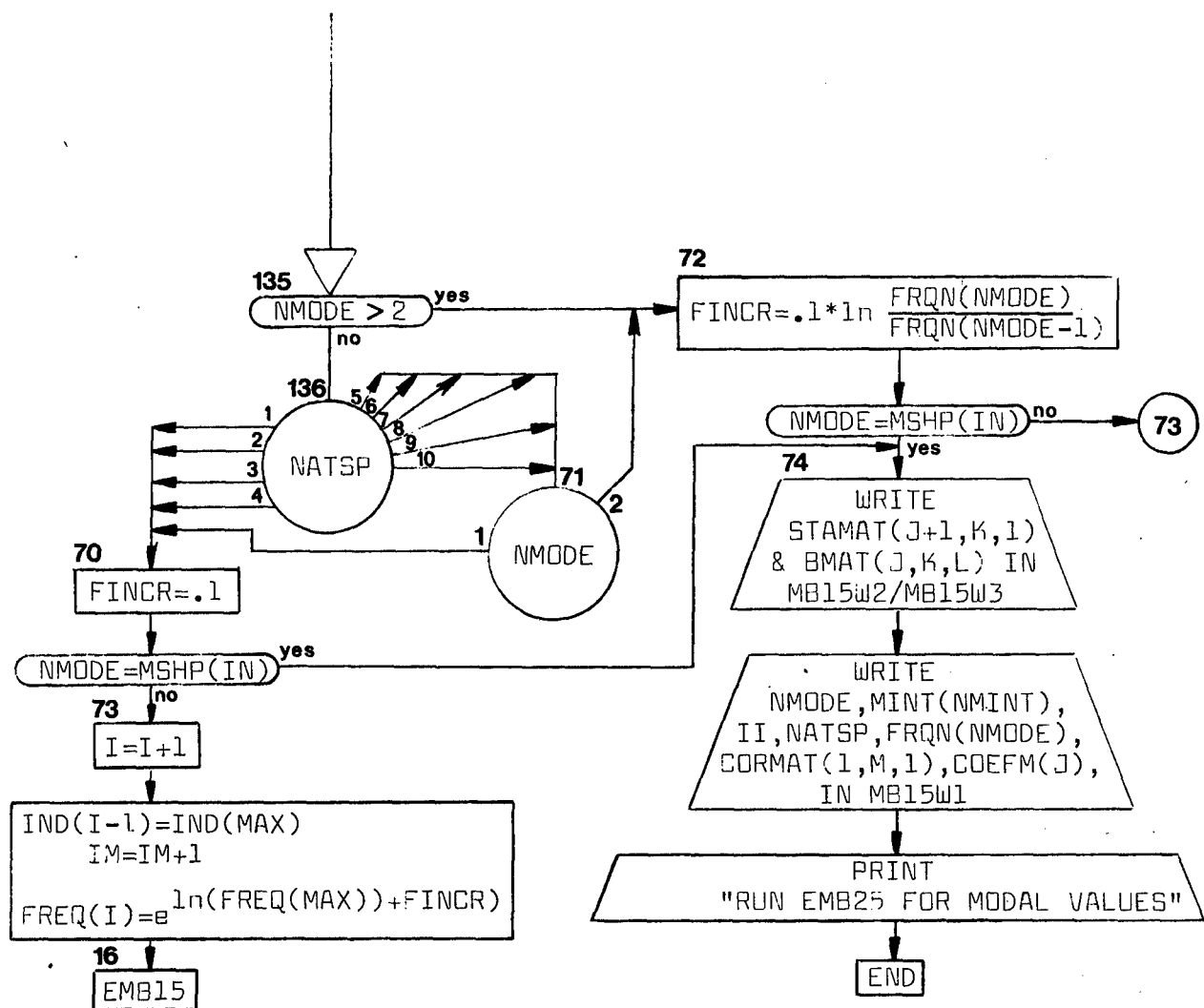
Saved Item EMB20

Figure 19

# Flow Chart of EMB20

Figure 20





MB15W2

10:46

TUE. 09/23/69.

STAMAT(1)  
.10000000E+01  
-.17750559E+00  
.00000000E+01  
.00000000E+01

STAMAT( 2)  
.10000000E+01  
-.17750559E+00  
.00000000E+01  
.00000000E+01

STAMAT( 3)  
.39661947E+00  
-.17734827E+00  
-.26156623E+00  
.12658088E+01

STAMAT( 4)  
.96282933E-01  
-.17475818E+00  
-.37194143E+01  
.81612458E+02

STAMAT( 5)  
.96282933E-01  
-.17475818E+00  
-.37194143E+01  
.12450087E+03

BMAT( 1)  
.10000000E+01 .00000000E+01 .00000000E+01 .00000000E+01  
.00000000E+01 .10000000E+01 .00000000E+01 .00000000E+01  
.00000000E+01 .00000000E+01 .10000000E+01 .00000000E+01  
.00000000E+01 .00000000E+01 .00000000E+01 .10000000E+01

BMAT( 2)  
.10001575E+01 .34001071E+01 .42815261E-03 .48523679E-03  
.18526679E-03 .10001575E+01 .25185978E-03 .42815261E-03  
.18239330E+00 .25011017E+01 .10001575E+01 .34001071E+01  
.12981846E+01 .18239330E+00 .18526679E-03 .10001575E+01

BMAT( 3)  
.10032907E+01 .17011188E+01 .10706052E-03 .60660020E-04  
.77433002E-02 .10032907E+01 .12600880E-03 .10706052E-03  
.32597351E+02 .10453455E+03 .10032907E+01 .17011188E+01  
.21714918E+03 .32597351E+02 .77433002E-02 .10032907E+01

BMAT( 4)  
.10000000E+01 .00000000E+01 .00000000E+01 .00000000E+01  
.00000000E+01 .10000000E+01 .00000000E+01 .00000000E+01  
.00000000E+01 .00000000E+01 .10000000E+01 .00000000E+01  
.44544149E+03 .00000000E+01 .00000000E+01 .10000000E+01

Portion of Permanent File MB15W2

Figure 21

MB15W1 10:45 TUE. 09/23/69.

NMØDE= 4 MINT(NMINT)= 5 II= 1 NATSP= 2 FRQN= .15672561E+03  
CØRMAT(1) 0.0 1.0 0.0 0.0  
CØEFM( 1)= .72925182E-01  
CØEFM( 2)= .31183327E+00  
CØEFM( 3)= .31183327E+00  
CØEFM( 4)= .31229857E+00  
CØEFM( 5)= .95974937E-01  
CØEFM( 6)= .92346649E-01  
CØEFM( 7)= .92346649E-01  
CØEFM( 8)= .71525685E-01  
CØEFM( 9)= .64630752E-01  
CØEFM(10)= .18062243E+00  
CØEFM(11)= .18062243E+00  
CØEFM(12)= .24705218E+00  
CØEFM(13)= .11119480E+01  
CØEFM(14)= .82500390E+00  
CØEFM(15)= .14667209E+00  
CØEFM(16)= .14667209E+00  
CØEFM(17)= .10371283E+00

Permanent File MB15W1

Figure 22

RUN

EMB15 13:09 WED. 09/03/69.

IN EMB20

IN EMB21

IN EMB20

SIZE AT LINE NO. 105, 3009, 1810, 0

INPUT VALUES OF II, IM, IN, NMØDE, FREQ(1), FINCR AND IND(1)

? ?1,2,2,2,5,...2,1

FRQN( 1)=

? ?0.

DIFFERENCE VALUE( 2)=	.21752718E+01	FREQ=	.61070138E+01
DIFFERENCE VALUE( 3)=	.31549069E+01	FREQ=	.74591235E+01
DIFFERENCE VALUE( 4)=	.43915608E+01	FREQ=	.91105939E+01
DIFFERENCE VALUE( 5)=	.57982241E+01	FREQ=	.11127705E+02
DIFFERENCE VALUE( 6)=	.70811595E+01	FREQ=	.13591409E+02
DIFFERENCE VALUE( 7)=	.75630462E+01	FREQ=	.16600584E+02
DIFFERENCE VALUE( 8)=	.59732824E+01	FREQ=	.20275999E+02
DIFFERENCE VALUE( 9)=	.35757613E+00	FREQ=	.24765162E+02
DIFFERENCE VALUE(10)=	-.11560836E+02	FREQ=	.30248236E+02

ROUGH FRQN( 2)= .24929665E+02= .39676701E+01HZ

ICRØSS= 9

DIFFERENCE VALUE(11)= .75885236E-01 FREQ= .24929665E+02  
ICRØSS= 10

DIFFERENCE VALUE(12)=	.15826825E-01	FREQ=	.24964343E+02
DIFFERENCE VALUE(13)=	.32953923E-02	FREQ=	.24971572E+02
DIFFERENCE VALUE(14)=	.66740138E-03	FREQ=	.24973075E+02
DIFFERENCE VALUE(15)=	.17710266E-03	FREQ=	.24973380E+02
DIFFERENCE VALUE(16)=	-.22733404E-04	FREQ=	.24973461E+02

ICRØSS= 15

DIFFERENCE VALUE(17)= .47282049E-04 FREQ= .24973452E+02  
ICRØSS= 16

DIFFERENCE VALUE(18)=	.14763141E-04	FREQ=	.24973458E+02
DIFFERENCE VALUE(19)=	.30237404E-05	FREQ=	.24973459E+02
DIFFERENCE VALUE(20)=	.59870047E-06	FREQ=	.24973459E+02

FINAL FRQN( 2)= .24973459E+02= .39746402E+01HZ INTERP= 10

RUN EMB25 FOR NODAL VALUES

Sample Output for Second Mode

Figure 23

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### Computation of Eigenmodes and Stress Values

Lines 1-57 represent the input section of EMB25 (refer to Figures 25 and 26), which is based on the initial data set EMB10R, the output files MB10W2 and MB10W3 of EMB10 and the output files of EMB15/20. The program computes  $(VAL(I,L), I=1,6), L=1, LSUM+NSS+2)$  representing the four components of the state vectors plus the maximum bending and average shear stresses at each of the 53 values of distance  $DIST(L)$  from the left-hand support. The beam as partitioned by the point loads and property jump discontinuities is shown in Figure 24, which also shows the 53 stations corresponding to the  $L$  values. At each discontinuity,  $L$  is incremented to account for shear force change due to the point load as well as for stress changes due to the possibility of variations in cross-section properties  $PAR(5-7,J)$ . The latter occur at  $L=22,23$  and  $L=37,38$ ; the remaining 10 discontinuities do not affect stress values as they correspond to changes in distributed load, material density or elastic modulus  $PAR(2-4,J)$ .  $VAL(I,L)$  are first computed on the basis of existing state vectors, i.e. the STAMAT values as read in. The largest deflection (in absolute value) is then found ( $VALMAX$ ) and compared with unity to form  $RATIO$ . The appropriate normalized component of  $Z_1$ , either  $W$  or  $V$ , is then multiplied by  $RATIO$  and a new series of state vectors obtained through the \$USE EMB21 monitor line. These renormalized values are returned to statement 119 and a new sequence of  $VAL(I,L)$  obtained which will contain a maximum displacement of  $\pm 1$  inch.

Lines 59-69 establish the number and length of the subspan segments,  $XNOSEG(J)$  and  $SUBSEG(J)$  respectively; a maximum of 20 segments is assured (line 50), but this may be varied by the user at will. For the given beam no subspan  $> SSUM/20$ , hence  $XNOSEG(J) = 2$  for all  $J$ . Line 71 establishes the limit of  $L$ ,  $LIM2$ , as the sum of the number of subspans  $NSS$ , the total number of segments  $LSUM$ , plus one station for each end of the beam. Lines 73-75 initialize  $VAL(I,L)$  using the state vector as read in or returned by EMB21. In line 81  $J=IPAR(1,K)$  means that  $VAL(I,L)$  are to be computed for

the right side of a point load, and if  $NSS < IPAR(1,K)$  lines 85-87 compute the  $VAL(I,LIM2)$  and control passes to statement 103. If  $J \neq IPAR(1,K)$  lines 93-97 compute the  $VAL(I,L)$  for the left side of a general subspan. Lines 99-147 are involved in the computation of  $VAL(I,L)$  for all the remaining stations within a subspan. SPAN is a summer of the segment lengths for each subspan and has a maximum of  $SS(J)$ . The value of  $L$  at the beginning of a subspan is retained in LSAVE, to which the current SPAN is added to obtain the corresponding  $DIST(L)$  (line 103). As long as  $M < LIM1$  control passes to statement 44 which considers the possibility of a massless field. The NO branch causes  $VAL(I,L)$  to be computed in accordance with Figure 3—the YES branch in accordance with Figure 2; control then passes to 42. When  $M=LIM1$  the  $VAL(I,L)$  are computed for the right side of the subspan and  $J$  is incremented for the next subspan/point load.

In lines 149-165 renormalization is performed as described above, involving the compilation of the additional 1000 characters of EMB21, producing a total length of about 5400 (about 5300 machine words). If further modes of interest remain the YES branch of line 177 is taken and a notice printed to rerun EMB15. If  $NMODE=MINT(NMINT) = MSHP(NMSHP)$  the possibility of further support conditions is investigated in statement 102; the NO branch signifies problem completion. The YES branch causes a rerun EMB15 notice to be printed.



```

1 $FILE EMB10R,MB10W2,MB10W3,MB15W1,MB15W2/MB15W3
3 DIMENSION NPAR(7),PAR(7,17),IPAR(1,20),SUBSEG(17),SS(17),
5 + XN0SEG(17),C0EFM(17),BMAT(23,4,4),STAMAT(24,4,1),C0RMAT(24,4,1),
7 + VAL(6,70),DIST(70),A(4),MINT(20)
9 READ(1,28) NSS,N0SUP,NMINT
11 28 F0RMAT(4X,3I5)
13 READ(1,29) (SS(I),I=1,NSS)
15 29 F0RMAT(4X,6F10.3)
17 READ(3,30) (NPAR(I),I=1,7)
19 30 F0RMAT(17X,7I3)
21 NPAR1=NPAR(1)
23 READ(3,31) (NPAR1,(IPAR(1,I),I=1,NPAR1)),SSUM
25 31 F0RMAT(/7X,*I4//33X,E15.8)
27 READ(2)
29 D0 32 I=1,7
31 NPR=NPAR(I)
33 32 READ(2,33) (PAR(I,J),J=1,NPR)
35 33 F0RMAT(6X,4E15.8)
37 READ(4,16) NM0DE,MINT(NMINT),II,NATSP,(C0RMAT(1,M,1),M=1,4),
39 + (C0EFM(J),J=1,NSS)
41 16 F0RMAT(6X,I2,14X,I2,5X,I2,8X,I2/9X,4F4.1/(11X,E15.8))
43 LIM=NSS+NPAR(1)
45 READ(5,18) (STAMAT(1,M,1),M=1,4); 18 F0RMAT(/(57X,E14.8))
47 D0 19 J=1,LIM
49 READ(5,35) A(1),A(2); 35 F0RMAT(A6/A6)
51 D0 19 K=1,4
53 READ(5,20) (BMAT(J,K,L),L=1,4),STAMAT(J+1,K,1)
55 20 F0RMAT(4E14.8,X,E14.8)
57 19 C0NTINUE
59 SEG=SSUM/20.; LSUM=ITER=0
61 D0 1 J=1,NSS
63 IF(2-FIX(AINT(SS(J)/SEG))) 13,10,10
65 10 XN0SEG(J)=2; G0 T0 17; 13 XN0SEG(J)=AINT(SS(J)/SEG)
67 17 LSUM=LSUM+FIX(XN0SEG(J))
69 1 SUBSEG(J)=SS(J)/XN0SEG(J)
71 LIM2=LSUM+NSS+2
73 119 D0 50 K=1,4; VAL(K,1)=STAMAT(1,K,1); VAL(5,1)=VAL(3,1)*
75 +PAR(7,1)/PAR(5,1); 50 VAL(6,1)=VAL(4,1)/PAR(6,1)
77 K=1; L=2; J=0; DIST(1)=0.
79 12 J=J+1; DIST(L)=DIST(L-1)
81 IF(J-IPAR(1,K)) 3,4,3; 4 IF(NSS-IPAR(1,K)) 9,2,2
83 2 D0 5 I=1,4; 5 VAL(I,L)=STAMAT(J+K,I,1)
85 VAL(5,L)=VAL(3,L)*PAR(7,J)/PAR(5,J)
87 VAL(6,L)=VAL(4,L)/PAR(6,J); K=K+1; G0 T0 8
89 9 D0 15 I=1,4; 15 VAL(I,L)=STAMAT(J+K,I,1)
91 VAL(5,L)=VAL(5,L-1); VAL(6,L)=VAL(4,L)/PAR(6,J-1); G0 T0 103
93 3 D0 14 I=1,4; 14 VAL(I,L)=STAMAT(J+K-1,I,1)
95 VAL(5,L)=VAL(3,L)*PAR(7,J)/PAR(5,J)
97 VAL(6,L)=VAL(4,L)/PAR(6,J)
99 8 SPAN=0.; LIM1=XN0SEG(J); LSAVE=L; M=0
101 EI=PAR(4,J)*PAR(5,J); 42 M=M+1; SPAN=SPAN+SUBSEG(J); L=L+1
103 DIST(L)=DIST(LSAVE)+SPAN
105 IF(LIM1-M) 44,40,44; 40 D0 43 I=1,4; 43 VAL(I,L)=STAMAT(J+K,I,1)
107 VAL(5,L)=VAL(3,L)*PAR(7,J)/PAR(5,J); VAL(6,L)=VAL(4,L)/PAR(6,J)

```

Program EMB25

Figure 25

```

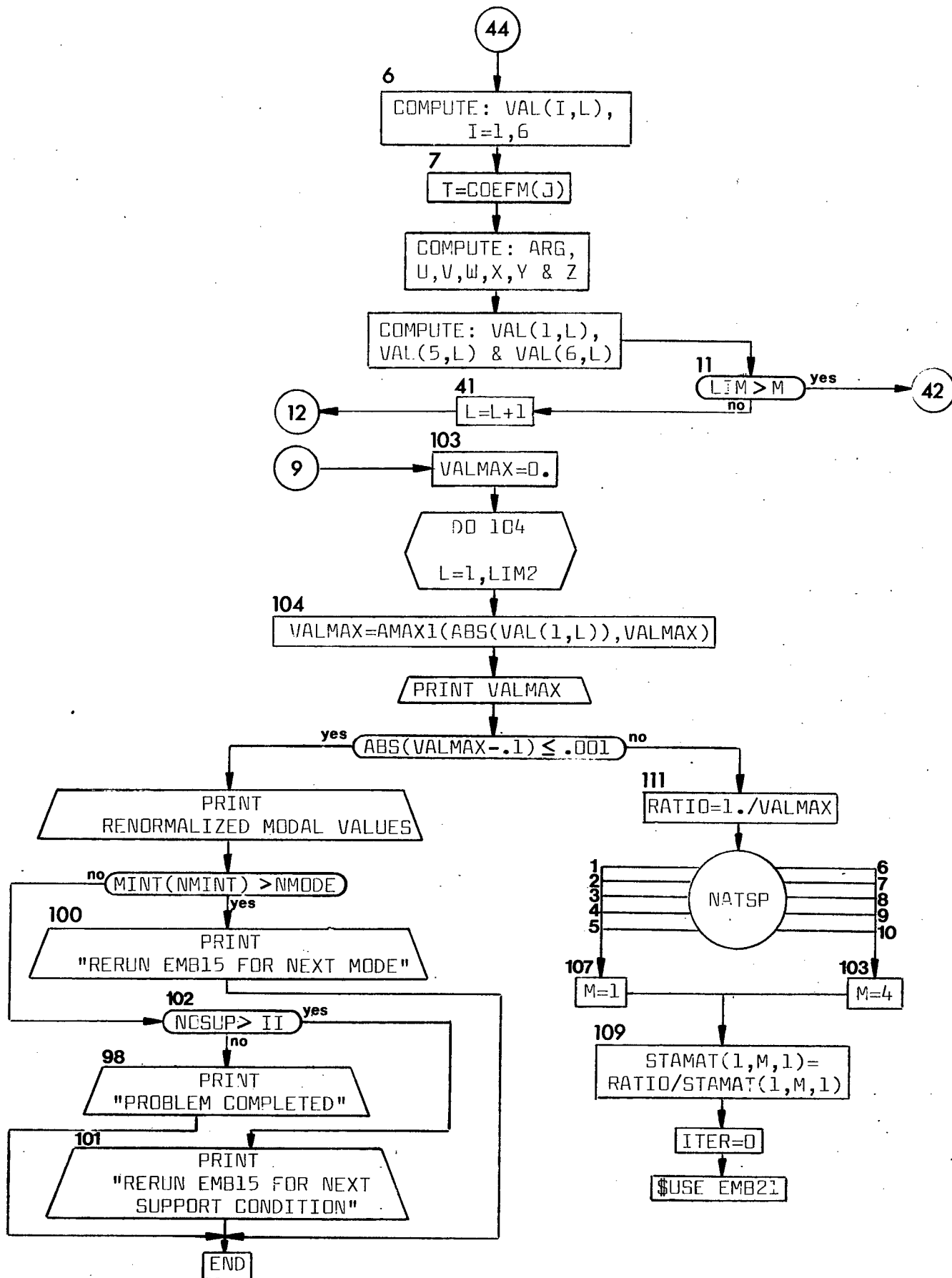
109 L=L+1; GØ TØ 12; 44 IF(PAR(3,J)-.0001) 6,6,7
111 6 VAL(1,L)=VAL(1,L-1)+VAL(2,L-1)*SPAN
113 + +VAL(3,L-1)*SPAN**2/(2.*EI)+VAL(4,L-1)*SPAN**3/(6.*EI)
115 VAL(2,L)=VAL(2,L-1)+VAL(3,L-1)*SPAN/EI+VAL(4,L-1)*SPAN**2/(2.*EI)
117 VAL(3,L)=VAL(3,L-1)+VAL(4,L-1)*SPAN
119 VAL(4,L)=VAL(4,L-1); VAL(5,L)=VAL(3,L)*PAR(7,J)/PAR(5,J)
121 VAL(6,L)=VAL(4,L)/PAR(6,J); GØ TØ 11
123 7 T=CØEFM(J); ARG=T*SPAN
125 U=(EXP(ARG)+EXP(-ARG))/2.; V=CØS(ARG)
127 X=(EXP(ARG)-EXP(-ARG))/2.; Y=SIN(ARG)
129 W=U-V; Z=X-Y
131 VAL(1,L)=.5*(VAL(1,LSAVE)*(U+V)+VAL(2,LSAVE)*(X+Y)/T
133 + +VAL(3,LSAVE)*W/(EI*T**2)+VAL(4,LSAVE)*Z/(EI*T**3))
135 VAL(5,L)=.5*(VAL(1,LSAVE)*W*EI*T**2+VAL(2,LSAVE)*Z*EI*T
137 + +VAL(3,LSAVE)*(U+V)+VAL(4,LSAVE)*(X+Y)/T)*PAR(7,J)/PAR(5,J)
139 VAL(6,L)=.5*(VAL(1,LSAVE)*(X+Y)*EI*T**3+VAL(2,LSAVE)*W*EI*T**2
141 + +VAL(3,LSAVE)*Z*T+VAL(4,LSAVE)*(U+V))/PAR(6,J)
143 11 IF(LIM1-M) 41,41,42
145 41 L=L+1
147 GØ TØ 12
149 103 VALMAX=0.; DØ 104 L=1,LIM2
151 104 VALMAX=AMAX1(ABS(VAL(1,L)),VALMAX)
153 PRINT 177,VALMAX; 177 FØRMAT(7HVALMAX=,E15.8)
155 IF(ABS(ABS(VALMAX)-1.)-.001) 110,110,111
157 111 RATIO=1./VALMAX
159 GØ TØ(107,107,107,107,107,108,108,108,108,108), NATSP
161 107 M=1; GØ TØ 109; 108 M=4
163 109 STAMAT(1,M,1)=RATIO*STAMAT(1,M,1); ITER=0
165 $USE EMB21
167 110 PRINT 178, "RENØORMALIZED MØDAL VALUES FØR NMØDE=",NMØDE
169 178 FØRMAT(I2/7IHDIST ALØNG BEAM DEFLECTION BENDING
171 + STRESS SHEAR STRESS)
173 PRINT 179, (DIST(L),VAL(1,L),VAL(5,L),VAL(6,L),L=1,LIM2)
175 179 FØRMAT(E15.8,4X,E15.8,4X,E15.8,3X,E15.8)
177 IF(MINT(NMINT)-NMØDE)102,102,100
179 100 PRINT,"RERUN EMB15 FØR NEXT MØDE"
181 GØ TØ 99
183 102 IF(NØSUP-II) 98,98,101; 98 PRINT~"PRØBLEM CØPLETED"; GØ TØ 99
185 101 PRINT~"RERUN EMB15 FØR NEXT SUPPORT CØNDITION"
187 99 CØNTINUE
189 $ØPT SIZE
191 END

```

LENGTH  
 ABOUT 4400 CHARS.

## Figure 26





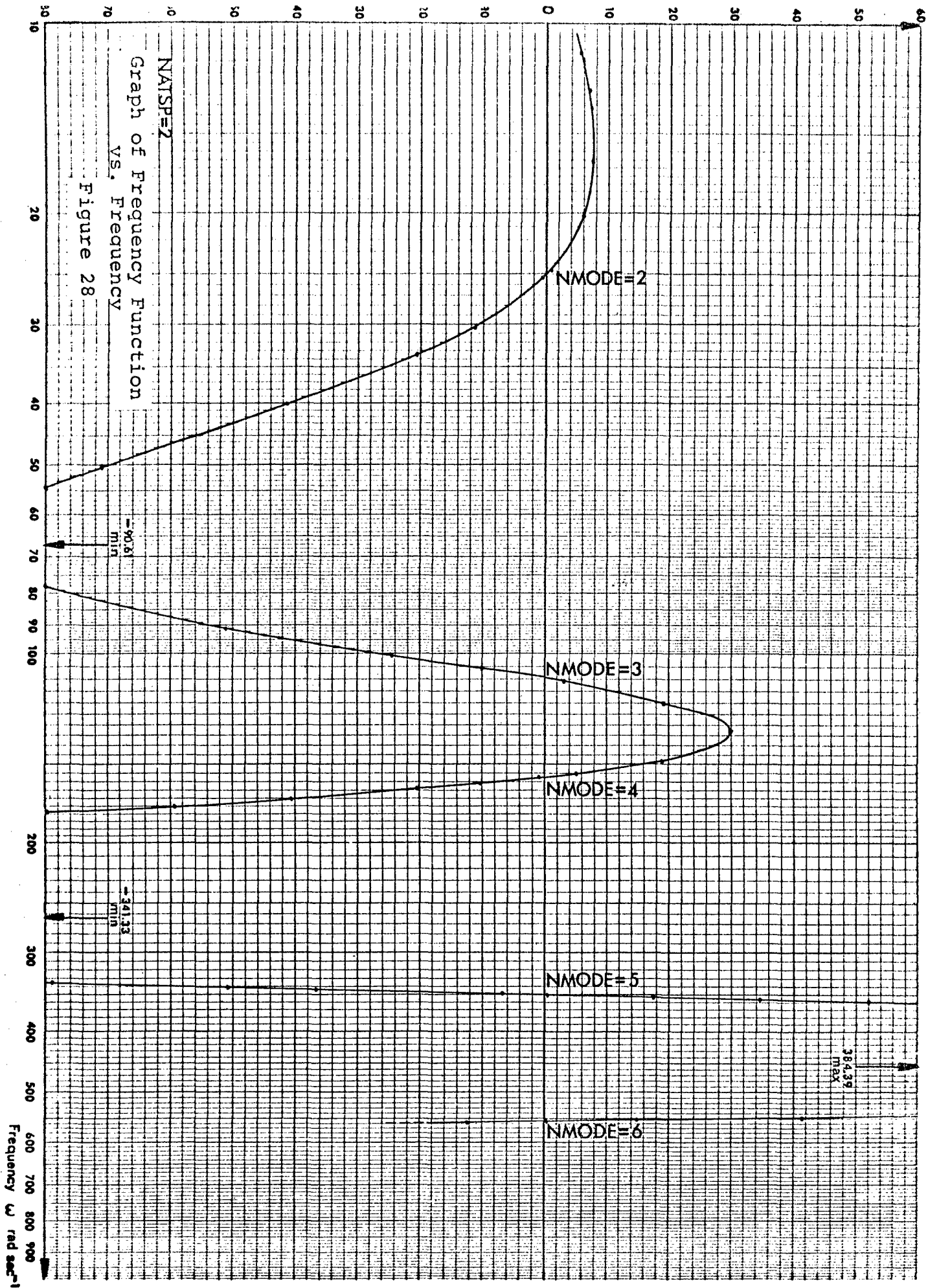
Eigenmode and Stress  
Values for Second Mode

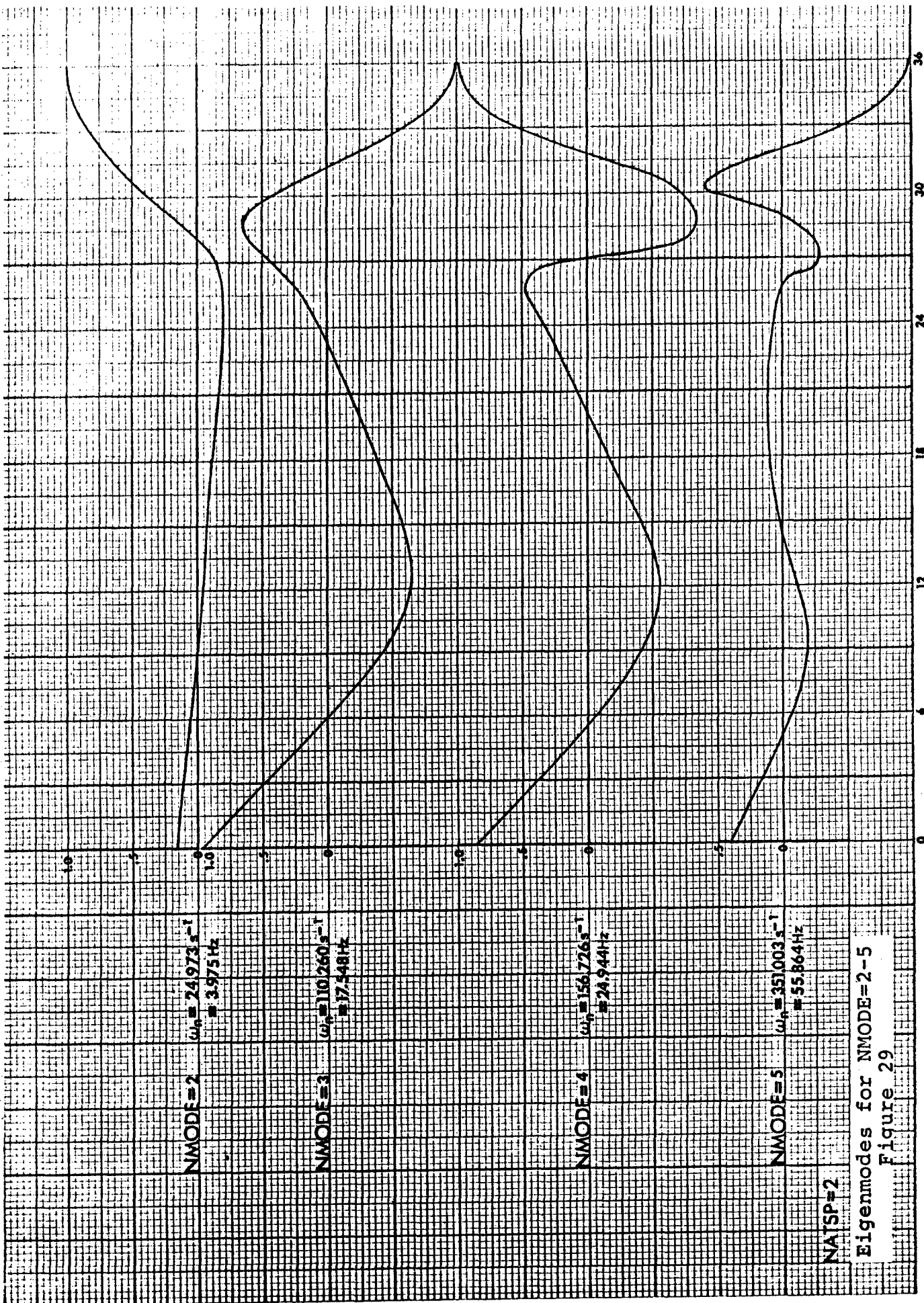
Figure 27.

RENORMALIZED MODAL VALUES FOR NMØDE= 2			
DIST ALØNG BEAM	DEFLECTION	BENDING STRESS	SHEAR STRESS
.00000000E+01	.17096813E+00	.00000000E+01	.00000000E+01
.00000000E+01	.17096813E+00	.00000000E+01	.00000000E+01
.17000000E+01	.13863570E+00	.74800825E+00	.42520182E-01
.34000000E+01	.10630383E+00	-.29724837E+00	.93335385E-01
.34000000E+01	.10630383E+00	-.29724837E+00	.93335385E-01
.42500000E+01	.90138766E-01	.40675727E+02	.46033042E+01
.51000000E+01	.73981174E-01	-.22423784E+01	.97509838E+01
.51000000E+01	.73981174E-01	-.22423784E+01	.23696611E+02
.61500000E+01	.54053567E-01	.53548799E+03	.27327400E+02
.72000000E+01	.34258465E-01	.97672862E+03	.31896800E+02
.72000000E+01	.34258465E-01	.97672862E+03	.31896800E+02
.84000000E+01	.11963786E-01	.17632168E+04	.33401524E+02
.96000000E+01	-.97661641E-02	.24719614E+04	.36110421E+02
.96000000E+01	-.97661641E-02	.24719614E+04	.36110421E+02
.10300000E+02	-.22083538E-01	.29774743E+04	.36105009E+02
.11000000E+02	-.34076703E-01	.34829734E+04	.36103775E+02
.11000000E+02	-.34076703E-01	.34829734E+04	.36103775E+02
.11450000E+02	-.41601325E-01	.38078715E+04	.36095519E+02
.11900000E+02	-.48979073E-01	.41328200E+04	.36088912E+02
.11900000E+02	-.48979073E-01	.41328200E+04	.32132053E+02
.13400000E+02	-.72358416E-01	.50961606E+04	.32087821E+02
.14900000E+02	-.93553749E-01	.60599847E+04	.32061441E+02
.14900000E+02	-.93553749E-01	.87235864E+03	.76947458E+01
.16600000E+02	-.11602906E+00	.10284320E+04	.76083735E+01
.18300000E+02	-.13816474E+00	.11855985E+04	.75397028E+01
.18300000E+02	-.13816474E+00	.11855985E+04	.75397028E+01
.18700000E+02	-.14331823E+00	.12217342E+04	.75215105E+01
.19100000E+02	-.14844937E+00	.12579511E+04	.75039793E+01
.19100000E+02	-.14844937E+00	.12579511E+04	.75039793E+01
.20450000E+02	-.16559389E+00	.13462522E+04	.33248485E+01
.21800000E+02	-.18245923E+00	.14828485E+04	-.44686274E+00
.21800000E+02	-.18245923E+00	.14828485E+04	-.51637540E+01
.22850000E+02	-.19537445E+00	.13934587E+04	-.90741855E+01
.23900000E+02	-.20811463E+00	.13434725E+04	-.12738841E+02
.23900000E+02	-.20811463E+00	.13434725E+04	-.12738841E+02
.24950000E+02	-.22048921E+00	.11553423E+04	-.17175322E+02
.26000000E+02	-.23235639E+00	.10107633E+04	-.21374701E+02
.26000000E+02	-.23235639E+00	.84257230E+05	-.35624502E+03
.26550000E+02	-.21418935E+00	.71825209E+05	-.39666836E+03
.27100000E+02	-.15263101E+00	.60441613E+05	-.42806465E+03
.27100000E+02	-.15263101E+00	.60441613E+05	-.42806465E+03
.28650000E+02	.10843820E+00	.18985358E+05	-.44156728E+03
.30200000E+02	.39777000E+00	-.78795382E+04	-.46541155E+03
.30200000E+02	.39777000E+00	-.78795382E+04	-.46541155E+03
.31150000E+02	.56774915E+00	-.34404013E+05	-.46526305E+03
.32100000E+02	.71891080E+00	-.60931984E+05	-.46516810E+03
.32100000E+02	.71891080E+00	-.60931984E+05	-.54045866E+00
.32950000E+02	.82807431E+00	-.60954241E+05	-.32738883E+00
.33800000E+02	.91054772E+00	-.60984404E+05	-.15306567E+00
.33800000E+02	.91054772E+00	-.60984404E+05	-.15306567E+00
.34900000E+02	.97764322E+00	-.60991760E+05	-.68837025E-01
.36000000E+02	.10000119E+01	-.61003762E+05	.39500943E-02
.36000000E+02	.10000119E+01	-.61003762E+05	.39500943E-02

RERUN EMB15 FOR NEXT MØDE







Eigenmodes for NMODE=2-5

Figure 29

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